

🗘 2021 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES.

# LAND DEVELOPMENT PLANS FOR PLANNING AND ZONING **SPECIAL PERMIT APPLICATION**

# **PROPOSED RETAIL DEVELOPMENT**

**1100 BOSTON TURNPIKE BOLTON, CONNECTICUT** 

PREPARED FOR: GARRETT HOMES, LLC **59 FIELD STREET** TORRINGTON, CT 06790

PREPARED BY:



ARCHITECTURE ENGINEERING ENVIRONMENTAL LAND SURVEYING

100 CONSTITUTION PLAZA, 10TH FLOOR HARTFORD, CONNECTICUT 06103 (860) 249-2200 (860) 249-2400 Fax

FOR PERMITTING PURPOSES ONLY NOT RELEASED FOR CONSTRUCTION



VICINITY MAP

SCALE: 1"=800'

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED DATE OF EXPIRATION

CHAIRMAN THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMNTS EXPIRES ON

DEVELOPER: GARRETT HOMES, LLC **59 FIELD STREET** TORRINGTON, CT 06790

### OWNER:

1100 BOSTON TRUNPIKE LLC C/O JOEL ROSENLICHT 483 MIDDLE TURNPIKE WEST, SUITE 102 MANCHESTER, CT 06040

# DATES

ISSUE DATE: **REVISION:** 

APRIL 2, 2021 MAY 5, 2021

(REVISED PER TOWN COMMENTS)



		Million 1999 1997 1997 1997 1997 1997 1997 199		
	SCRIPTION — 1100 f an existing 5/8" rod in the n	BOSTON TU NORTHERIX HIGHWA	JRNPIKE: V LINE OF BT 44 ALSO K	
BOSTON TURNPIK SOUTHWEST COR	E, MARKING THE SOUTHEAST CO NER OF LAND NOW OR FORMAL	RNER OF THE PROPE	RTY BEING DESCRIBED	HEREIN AND THE
HIGHWAY LINE OI SOUTHEASTERN C	F SAID BOSTON TURNPIKE S82°24	1'31"W, 410.30' TO A 1ERLY OF LORETTA G	N EXISTING 5/8" ROD I	MARKING THE OUTHWESTERLY
CORNER OF LAND	D BEING DESCRIBED HEREIN; THE D CHARLES ROSE, AND SARAH CH	NCE ALONG LANDS C	F HARRY AND ANDREV	V HOAR, ANDREW 515.27 TO A POINT
MARKING NORTH CORNER OF LAND	EAST CORNER OF LAND NOW OR BEING DESCRIBED HEREIN; THE	FORMERLY OF SARA	H CHAMBERLAIN AND THERLY LINE OF LAND I	THE NORTHWEST NOW OR
FORMERLY OF ST	ATE OF CONNECTICUT THE FOLLO	WING FIVE COURSE	:S: 16'43"F 70 26' N76°42	2'47"F 203 86 TO
A POINT MARKING	G THE NORTHWEST CORNER OF I	AND NOW OR FORM	IERLY OF MISSIONARY	SOCIETY FOR THE
MISSIONARY FOR CORNER OF SAID	THE DIOCESE OF CONNECTICUT	547°34'01"E, 275.82′, G SAID CH HOLDINGS	, TO A POINT MARKING LLC S82°24'31"W. 361	THE NORTHEAST
MARKING THE NC \$07°35'29"E, 261.	ORTHWEST CORNER OF SAID CH F .01', TO AN EXISTING %" ROD; TH	IOLDING LLC, THENC	E ALONG SAID CH HOLD	DING LLC C S67°35'29"E,
57.75', TO THE PC	DINT AND PLACE OF BEGINNING (	CONTAINING 236,912	34 OR 5.44 ACRES.	
	N/F			
THE 1	MISSINOARY SOCIETY FOR DIOCESE OF CONNECTICU	₹ THE T		
	VOL.34- PG.260			
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E AGENCIES SECTIONS 20- SURVEYS AND MAPS IN TH	-300b-20 AND THE F STATE OF CONNECTICUT" AS	FIRST AL	MERICAN TITLE INSURA MENT FOR TITLE INSUR	ANCE COMPANY -
SONNECTICUT ASSOCIATION SEPTEMBER 26, 1996.	OF LAND SURVEYORS,	COMMIT	ALNI NO. CIST42976 - D TO: DEVELOPMENT LLC	- DATED JANURARY 8, 2021
Y = PROPERTY SURVEY		FIRST A	MERICAN TITLE INSURA	ANCE COMPANY
$RMINATION \ CATEGORY = D$	EPENDENT RE-SURVEY	IS BASE	HS MAP OR PLAT AND D WERE MADE IN ACC	O THE SURVEY ON WHICH IT
ORD - URNPIKE LLC (Vol. 141- P	g 790)		CSM LAND TITLE SURV	EQUIREMENTS FOR EYS, JOINTLY ESTABISHED
236912 S.F. or 5.439 Ac.		A ITEMS	2,3,4,5,8,11,13,16,17,1	18 AND 19.
		SCHEDUI 1. NON-	LE B, PART II, EXCEPT -SURVEY ISSUE	FIONS:
SED ON NAVD 88 DATUM (I	MAP REFERENCE #2)	2. NON 3. PLOT 4. NON	SURVEY ISSUE TED SURVEY ISSUE	
F RECENT EARTH MOVING V IONS OBSERVED,	NORK, BUILDING CONSTRUCTION	5. NON 6. NON	SURVEY ISSUE SURVEY ISSUE	
N OF PROPOSED CHANGES	IN STREET RIGHT OF WAY	7. NON 8. NON	SURVEY ISSUE SURVEY ISSUE	
LK CONSTRUCTION OR REP	AIRS OBSERVED.	9. PLOT 10. PLOT 11. PLOT	TED NOTE T2 TTED TTED	
ELINEATION OBSERVED		12. PLO	TTED	
WITHIN FLOOD ZONE X	AREA OF MINIMAL FLOOD - FLOOD INSURANCE RATE - PANEL 1 OF	FIELD W	ORK WAS COMPLETED	ON FEBRUARY 17, 2021
EL NUMBER 090109 0001 E PREPARED BY FEDERAL EM	B WITH AN EFFECTIVE DATE OF MERGENCY MANAGEMENT			$\gamma$ h <
		DATE:	5/3/2021 CARMINI	E J. MATRASCIA - LS#70219
72 - PG 443 FOR POSSIB L 4, 1990 - EXEPTION #9	LE EFFECTS OF SPECIAL PERMI			
UTILITIES, STRUCTURE AND N HAVE BEEN COMPILED, I	D FACILITY LOCATIONS DEPICTED N PART, FROM RECORD	)	•	
BY THE RESPECTIVE UTILI SENCIES, FROM PAROLE TES	TY COMPANIES OR STIMONY AND FROM OTHER	2		
LOCATIONS ARE APPROXIMA WN TO DUFOUR SURVEYING STENCE OF ALL SUCH FEAT	ASSOCIATES. THE SIZE,	5		
VERIFIED BY THE APPROPRIALL BEFORE YOU DIG 1-800	ATE AUTHORITIES PRIOR TO	REVISED 3/31/2021 · 4	DD WETLANDS LIMITS	
		REVISED 3/22/2021 : C	ORRECTED ZONE RMUZ	
EF, THIS MAP IS SUBSTANTIALLY	$n \mathbb{N} \mathbb{E} \mathbb{O} / n$	ALTA /	'NSPS LAND	TITLE SURVEY
MTH THE STANDARDS OF A CLASS				
INED IN THE CODE OF PRACTICE FOR F SURVEYS AND MAPS, ADOPTED	,	THEFARED FOR:	KE. RTF 44 ROLTON	INT, LLC
BY THE CONNECTICUT		SCALE: 1" = 40'	APPROVED:	CARMINE J. MATRASCIA – L.S. #70219
		DATE: 02-18-2021	JOB NO.: 21-	05 FILE NO.: \21-05
ns			DUFOUR SURVEYIN	NG LLC
TRASCIA - L.S. #70219	"EYING		575 NORTH MAIN S BRISTOL, CONNECT 860-314-0502 860-	TREET NCUT 738-0222



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2 - LOT 8	SUBDIVISION	PLAN	
PREPARED FOR: CALITTO DEV	ELOPMENT LLC		n an
1100 BOSTON TURNPIKE, R	OUTE 44, BOLTON, CO		tana ang tanàng terang Na mang tanàng tanàng taona ang
SCALE: 1" = 40'	APPROVED: CARMINE J. M	MATRASCIA - L.S. #	70219
DATE: 03-18-2021	JOB NO.: 21-05	FILE NO.: \21-05	5
DU 5 86	FOUR SURVEYING LLC 75 NORTH MAIN STREET BRISTOL, CONNECTICUT 60-314-0502 860-738-0222		

# ZONING INFORMATION

LOCATION: BOLTON, TOLLAND COUNTY, CONNECTICUT

MINIMUM REAR SETBACK

MAXIMUM BUILDING HEIGHT

MAXIMUM BUILDING COVERAGE

MAXIMUM IMPERVIOUS COVERAGE

ZONE: RURAL MIXED USE ZONE (RMUZ)

USE: RETAIL (PERMITTED BY SPECIAL PERMIT)						
ITEM #	ITEM	REQUIREMENTS	PROPOSED LOT 3	FUTURE LOT 2		
1	MINIMUM LOT AREA	80,000 S.F.	80,707 S.F. (1.85 AC.)	82,061 S.F. (1.88 AC.) [2]		
2	MINIMUM LOT WIDTH	NONE REQUIRED	308 FEET	560 FEET		
3	MINIMUM LOT FRONTAGE	150 FEET	260.4 FEET	150 FEET		
4	MINIMUM FRONT SETBACK	NONE REQUIRED	71.9 FEET	343 FEET		
5	MINIMUM SIDE SETBACK	25 FEET (50 FEET) [1]	72.8 FEET	118.6 FEET		

1	ITEM #	
	1	
	_	
	2	

VARIANCE

NO

NO

NO

NO

NO

NO

NO

NO

NO

89.3 FEET

<35 FEET/2.5 STORIES

12.2 PERCENT

33.5 PERCENT

51.4 FEET

25.6 FEET

13.2 PERCENT

39.9 PERCENT

ITEM #	ITEM	REQUIREMENTS	PROPOSED LOT 3	FUTURE LOT 2	VARIANCE
1	BUILDING SIZE	600 S.F.	10,640 S.F.	10,000 S.F.	NO
2	PARKING REQUIRED	RETAIL: MINIMUM – 2 SPACES PER 1,000 S.F. OF GFA (10,640/10,000 S.F.) MINIMUM REQUIRED = 22 / 20 SPACES	33 SPACES	49 SPACES	NO
		MAXIMUM - 5 SPACES PER 1,000 S.F. OF GFA (10,640/10,000 S.F.) MAXIMUM ALLOWED = 54 / 50 SPACES			
3	MINIMUM HANDICAPPED PARKING SPACES REQUIRED	2 SPACES	2 SPACES	2 SPACES	NO
4	MINIMUM PARKING DIMENSIONS	9 FEET X 18 FEET	9 FEET X 20 FEET	9 FEET X 18 FEET	NO
5	MINIMUM LOADING DIMENSIONS	10 FEET X 25 FEET X 14 FEET	33 FEET X 71 FEET X > 14 FEET	10 FEET X 25 FEET X > 14 FEET	NO
6	MINIMUM AISLE WIDTH	22 FEET – 2–WAY 11 FEET – 1–WAY 11	30 FEET - 2-WAY	24 FEET - 2-WAY	NO
7	MINIMUM FRONT SETBACK	50 FEET [3]	50.5 FEET	273.4 FEET	NO
8	MINIMUM SIDE SETBACK	NONE REQUIRED [3]	77.1 FEET	66.7 FEET	NO
9	MINIMUM REAR SETBACK	NONE REQUIRED [3]	5.3 FEET	124 FEET	NO
10	BICYCLE PARKING REQUIRED	1 BICYLE PARKING SPACE PER 25 PARKING STALLS (2 REQUIRED)	2 BICYCLE PARKING SPACES	2 BICYCLE PARKING SPACES	NO

[1] MINIMUM SIDE AND REAR SETBACKS – 50 FEET WHEN ABUTTING A RESIDENTIAL DISTRICT [2] LOT AREA FOR LOT 2 DOES NOT INCLUDE ACCESS STRIP, CONSERVATION EASEMENT, OR WETLAND AREAS.

25 FEET (50 FEET) [1]

35 FEET/2.5 STORIES

25 PERCENT

50 PERCENT

# SITE PLAN LEGEND



6

7

8

9

EXISTING EASEMENT AREA

PROPERTY LINE

PROPOSED EASEMENT AREA

N/F CHAMBERLAIN SARAH VOL.176- PG.1153 1084 BOSTON TPKE

# SCREENING VEGETATION

ENCROACHMENT SHED EXEPTION #3 PROPOSED SUBDIVISION PROPERTY -LINE N/F ROSE CHARLES & MARIE VOL.41- PG.95 15 NORTH RD

> N/F HOAR ANDREW VOL.53- PG.792 11 NORTH RD

# EARTHEN BERM TO REMAIN AND BE PLANTED WITH SCREENING VEGETATION

N/F HOAR ANDRÉW & HARRY VOL.182- PG.1074 7 NORTH RD

N/F HOAR LORÉTTA GRACE VOL.147- PG.102 1084 BOSTON TPKE

PROPOSED 20' WDE SANITARY SEWER EASEMENT IN FAVOR OF THE BLRWPCA 

BOLTON PLANNING AND ZONING COMMISSION, B	OLTON, CT

DATE APPROVED DATE OF EXPIRATION

CHAIRMAN

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON

. COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANI



1. 2.	THESE PLANS ARE FOR PERMITTING PURPOSES ONLY AND ARE NOT FOR CONSTRUCTION. ALL CONSTRUCTION SHALL COMPLY WITH THE PROJECT SPECIFICATION MANUAL; CLIENT CORPORATION STANDARDS, MUNICIPALITY STANDARDS AND SPECIFICATIONS, CONNECTICUT DEPARTMENT OF TRANSPORTATION STANDARDS AND SPECIFICATIONS, 2010 ADA STANDARDS, AND STATE BUILDING CODE IN THE ABOVE REFERENCED INCREASING HIERARCHY. IF SPECIFICATIONS ARE IN CONFLICT, THE MORE STRINGENT SPECIFICATION SHALL APPLY.	47.	APPROVED OFF SITE L THE CONTRACTOR SHA THE CONTRACTOR SHA PERMITS AND DISPOSA
3.	ALL CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE OSHA, FEDERAL, STATE AND LOCAL REGULATIONS. REFER TO OTHER PLANS BY OTHER DISCIPLINES, DETAILS AND PROJECT MANUAL FOR ADDITIONAL INFORMATION. THE CONTRACTOR SHALL VERIFY ALL SITE AND BUILDING CONDITIONS IN THE FIELD AND CONTACT THE CIVIL ENGINEER AND ARCHITECT IF THERE ARE ANY QUESTIONS OR CONFLICTS REGARDING THE CONSTRUCTION DOCUMENTS AND/OR FIELD CONDITIONS, SO THAT APPROPRIATE REVISIONS CAN BE MADE PRIOR TO BIDDING. ANY	48. 49.	ASBESTOS OR HAZARE THE CONTRACTOR SHA
4.	CONFLICT BETWEEN THE DRAWINGS AND SPECIFICATIONS SHALL BE CONFIRMED WITH THE OWNER'S CONSTRUCTION MANAGER PRIOR TO BIDDING. DO NOT INTERRUPT EXISTING UTILITIES SERVICING FACILITIES OCCUPIED AND USED BY THE OWNER OR OTHERS DURING OCCUPIED HOURS EXCEPT WHEN SUCH INTERRUPTIONS HAVE BEEN AUTHORIZED IN WRITING BY THE OWNER AND THE LOCAL MUNICIPALITIES. INTERRUPTIONS SHALL ONLY OCCUR AFTER ACCEPTABLE TEMPORARY SERVICE HAS BEEN PROVIDED.	50.	STREET LINE OR AT TO SERVICES MAY NOT BE SERVICE PIPING TO BE
5.	THE CONTRACTOR SHALL ABIDE BY ALL OSHA, FEDERAL, STATE, AND LOCAL REGULATIONS WHEN OPERATING CRANES, BOOMS, HOISTS, ETC. IN CLOSE PROXIMITY TO OVERHEAD ELECTRIC LINES. IF CONTRACTOR MUST OPERATE EQUIPMENT CLOSE TO ELECTRIC LINES, CONTACT POWER COMPANY TO MAKE ARRANGEMENTS FOR PROPER SAFEGUARDS. ANY UTILITY COMPANY FEES SHALL BE PAID FOR BY THE CONTRACTOR.	51.	THE CONTRACTOR SHA CONTRACTOR DISTURB OF THE CONTRACTOR.
6. 7	THE CONTRACTOR SHALL PROVIDE AS-BUILT RECORD DRAWINGS OF ALL CONSTRUCTION (INCLUDING UNDERGROUND UTILITIES AND STORMWATER SYSTEM) TO THE OWNER AT THE END OF CONSTRUCTION.	52.	OR UNTIL THE COMMENT OR UNTIL THE COMMENT ENGINEER OR OWNER'S SEDIMENTATION CONTR PLAN. AT THAT TIME.
7.	THE ARCHITECT OR ENGINEER IS NOT RESPONSIBLE FOR SITE SAFETT MEASURES TO BE EMPLOYED DURING CONSTRUCTION. THE ARCHITECT AND ENGINEER HAVE NO CONTRACTUAL DUTY TO CONTROL THE SAFEST METHODS OR MEANS OF THE WORK, JOB SITE RESPONSIBILITIES, SUPERVISION OR TO SUPERVISE SAFETY AND DOES NOT VOLUNTARILY ASSUME ANY SUCH DUTY OR RESPONSIBILITY. THE CONTRACTOR SHALL COMPLY WITH CER 29 PART 1926 FOR EXCAVATION. TRENCHING, AND TRENCH PROTECTION REQUIREMENTS.	53.	THE CONTRACTOR SHA DISPOSAL AREA BY A
9.	INFORMATION ON EXISTING UTILITIES AND STORM DRAINAGE SYSTEMS HAS BEEN COMPILED FROM AVAILABLE INFORMATION INCLUDING UTILITY COMPANY AND MUNICIPAL OR COUNTY OR STATE RECORD MAPS AND/OR FIELD SURVEY AND IS NOT GUARANTEED CORRECT OR COMPLETE. UTILITIES AND STORM DRAINAGE SYSTEMS ARE SHOWN TO ALERT THE CONTRACTOR TO THEIR PRESENCE AND THE CONTRACTOR IS SOLELY RESPONSIBLE FOR	54.	IF IMPACTED OR CONT SOIL AND NOTIFY THE LOCATION UNTIL FURT
	DETERMINING ACTUAL LOCATIONS AND ELEVATIONS OF ALL UNDERGROUND AND OVERHEAD UTILITIES AND STORM DRAINAGE SYSTEMS INCLUDING SERVICES. PRIOR TO DEMOLITION OR CONSTRUCTION, THE CONTRACTOR SHALL CONTACT [CT CALL BEFORE YOU DIG (CBYD)] [CT (800) 922-4455] OR AT 811 AND VERIFY ALL UTILITY AND STORM DRAINAGE SYSTEM LOCATIONS. THE CONTRACTOR SHALL EMPLOY THE USE OF A UTILITY LOCATING COMPANY TO PROVIDE SUBSURFACE UTILITY ENGINEERING CONSISTING OF DESIGNATING UTILITIES AND STORM PIPING ON PRIVATE PROPERTY WITHIN	55.	EXISTING WATER SERV AUTHORITY. REMOVE OR AS REQUIRED BY
10.	THE CONTRACT LIMIT AND CONSISTING OF DESIGNATING AND LOCATING WHERE PROPOSED UTILITIES AND STORM PIPING CROSS EXISTING UTILITIES AND STORM PIPING WITHIN THE CONTRACT LIMITS. DO NOT SCALE DRAWINGS. DIMENSIONS GOVERN OVER SCALED DIMENSIONS.	56.	EXISTING SANITARY LA SANITARY UTILITY PRO REQUIRED BY THE SA
11. 12.	IF PLANS AND OR SPECIFICATIONS ARE IN CONFLICT, THE MOST COSTLY SHALL APPLY. ALL CONTRACTORS AND SUBCONTRACTORS SHALL OBTAIN COMPLETE DRAWING PLAN SETS FOR BIDDING AND CONSTRUCTION. PLAN SETS OR PLAN	57.	REQUIREMENTS. WORK TANKS SHALL BE PUF
	SET ELECTRONIC POSTINGS SHALL NOT BE DISASSEMBLED INTO PARTIAL PLAN SETS FOR USE BY CONTRACTORS AND SUBCONTRACTORS OF INDIVIDUAL TRADES. IT SHALL BE THE CONTRACTOR'S AND SUBCONTRACTOR'S RESPONSIBILITY TO OBTAIN COMPLETE PLAN SETS OR COMPLETE PLAN SET ELECTRONIC POSTINGS FOR USE IN BIDDING AND CONSTRUCTION.	58.	The contractor SH Provider, Gas Utilit Prior to Beginning
13. 14.	ALL NOTES AND DIMENSIONS DESIGNATED "TYPICAL" APPLY TO ALL LIKE OR SIMILAR CONDITIONS THROUGHOUT THE PROJECT. CONTRACTOR(S) TO TAKE AND VERIFY ALL DIMENSIONS AND CONDITIONS OF THE WORK AND BE RESPONSIBLE FOR COORDINATION OF SAME. FIELD	59. 60.	THE CONTRACTOR IS REQUIRED APPLICATIO BACK FILL DEPRESSIO
15.	BL COMPANIES WILL PREPARE FINAL CONSTRUCTION DOCUMENTS SUITABLE FOR BIDDING AND CONSTRUCTION. PROGRESS SETS OF THESE DOCUMENTS ARE NOT SUITABLE FOR THOSE PURPOSES. IF CLIENT ELECTS TO SOLICIT BIDS OR ENTER INTO CONSTRUCTION CONTRACTS UTILIZING CONSTRUCTION DOCUMENTS THAT ARE NOT YET FINAL, CONSULTANT SHALL NOT BE RESPONSIBLE FOR ANY COSTS OR DELAY ARISING AS A RESULT.		MATERIAL APPROVED FURTHER SITE CONST MATERIAL SPECIFIED I DRY DENSITY PER AS EQUIPMENT FOR DUST
16. 17.	NO CONSTRUCTION OR DEMOLITION SHALL BEGIN UNTIL APPROVAL OF THE FINAL PLANS IS GRANTED BY ALL GOVERNING AND REGULATORY AGENCIES. THE OWNER IS RESPONSIBLE FOR OBTAINING ALL NECESSARY ZONING PERMITS REQUIRED BY GOVERNMENT AGENCIES PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL CONTACT AND OBTAIN FROM COUNTY SOURCES ALL CONSTRUCTION PERMITS, INCLUDING ANY STATE DOT PERMITS, SEWER AND WATER CONVERTING AND DETAILS AND OBTAIN FROM COUNTY SOURCES ALL CONSTRUCTION PERMITS, INCLUDING ANY STATE DOT PERMITS, SEWER AND	61. 62	THE CONTRACTOR SH LOCAL GOVERNING AU
18.	WATER CONNECTION PERMITS, AND ROADWAY CONSTRUCTION PERMITS. THE CONTRACTOR SHALL POST ALL BONDS, PAY ALL FEES, PROVIDE PROOF OF INSURANCE AND PROVIDE TRAFFIC CONTROL NECESSARY FOR THIS WORK EXCEPT CTDOT ENCROACHMENT PERMIT BOND. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS OF ALL PRODUCTS AND MATERIALS PER PLANS AND SPECIFICATIONS TO THE OWNER AND CIVIL ENCINEER FOR REVIEW AND ADDROVAL DRIVE TO FARPICATION OF DELIVERY TO THE SITE. ALLOW A MINIMUM OF 14 WORKING DAYS FOR REVIEW	63	CONTRACTOR SHALL LIGHTING TO REMAIN
19. 20	THE CONTRACTOR SHALL FOLLOW THE SEQUENCE OF CONSTRUCTION NOTES PROVIDED ON THE SEDIMENT AND EROSION CONTROL PLAN.	64.	PERFORMED. THE CON THE CONTRACTOR SH
20. 21.	CONCRETE SIDEWALKS, LANDINGS, RAMPS, AND STAIRS. SHOULD ANY UNCHARTED OR INCORRECTLY CHARTED, EXISTING PIPING OR OTHER UTILITY BE UNCOVERED DURING EXCAVATION, CONSULT THE CIVIL	65.	REQUIRED. MAINTAIN INSTALLATION AND P. THE CONTRACTOR SH
22.	ALL SITE DIMENSIONS ARE REFERENCED TO THE FACE OF CURBS OR EDGE OF PAVING AS APPLICABLE UNLESS OTHERWISE NOTED. ALL BUILDING DIMENSIONS ARE REFERENCED TO THE OUTSIDE FACE OF THE STRUCTURE.	66.	THE CONTRACTOR OF BUILDINGS, STRUCTUF FNGINFFR, LICENSED
23.	THE CONTRACTOR SHALL PROVIDE AND MAINTAIN TRAFFIC DEVICES FOR PROTECTION OF VEHICLES AND PEDESTRIANS CONSISTING OF DRUMS, BARRIERS, SIGNS, LIGHTS, FENCES, TEMPORARY WALKWAYS, TRAFFIC CONTROLLERS AND UNIFORMED TRAFFIC OFFICERS AS REQUIRED OR AS ORDERED BY THE ENGINEER OR AS REQUIRED BY THE LOCAL GOVERNING AUTHORITIES OR AS REQUIRED BY PERMIT STIPULATIONS OR AS REQUIRED BY THE OWNER. CONTRACTOR SHALL MAINTAIN ALL TRAFFIC LANES AND PEDESTRIAN WALKWAYS FOR USE AT ALL TIMES UNLESS WRITTEN APPROVAL FROM	67. 68.	NO SALVAGE SHALL
24.	TRAFFIC CONTROL SIGNAGE SHALL CONFORM TO THE STATE DOT STANDARD DETAIL SHEETS AND THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES. SIGNS SHALL BE INSTALLED PLUMB WITH THE EDGE OF THE SIGN 2' OFF THE FACE OF THE CURB, AND WITH 7' VERTICAL CLEARANCE	69.	THE EXISTING DRIVEW
25.	UNLESS OTHERWISE DETAILED OR NOTED. REFER TO DETAIL SHEETS FOR PAVEMENT, CURBING, AND SIDEWALK INFORMATION.	70. 71.	TOPSOIL SHALL BE S
26. 27. 28.	THE CONTRACT LIMIT IS THE PROPERTY LINE UNLESS OTHERWISE SPECIFIED OR SHOWN ON THE CONTRACT DRAWINGS. THE CONTRACTOR SHALL SUBMIT A SHOP DRAWING OF THE PAVEMENT MARKING PAINT MIXTURE PRIOR TO STRIPING. PAVEMENT MARKING KEY:	72. 73.	SUBGRADE SHALL BE MATERIAL AS REQUIN THE CONTRACTOR SH
	4" SYDL 4' SOLID YELLOW DOUBLE LINE 4" SYL 4" SOLID YELLOW LINE 4" SWL 4" SOLID WHITE LINE	74.	AREAS TO 95% OF T ENGINEER. UNDERDRAINS SHALL
	12" SWSB 12" SOLID WHITE STOP BAR 4" BWL 4" BROKEN WHITE LINE 10' STRIPE 30' SPACE	75.	GRADED. VERTICAL DATUM IS
29. 30.	PARKING SPACES SHALL BE STRIPED WITH 4" SWL; HATCHED AREA SHALL BE STRIPED WITH 4"SWL AT A 45" ANGLE, 2' ON CENTER. HATCHING, SYMBOLS, AND STRIPING FOR HANDICAPPED SPACES SHALL BE PAINTED WHITE AND BLUE. OTHER MARKINGS SHALL BE PAINTED WHITE OR AS NOTED. ALL PARKING SPACES AND HATCHED AREAS SHALL HAVE TWO COATS OF PAVEMENT MARKINGS APPLIED TO STRIPING.	76.	CLEARING LIMITS SHA THE SITE.
31. 32.	PAVEMENT MARKINGS SHALL BE HOT APPLIED TYPE IN ACCORDANCE WITH STATE DOT SPECIFICATIONS, UNLESS WHERE EPOXY RESIN PAVEMENT MARKINGS ARE INDICATED. THE CONTRACTOR SHALL RESTORE ANY UTILITY STRUCTURE, DRAINAGE STRUCTURE, PIPE, UTILITY, PAVEMENT, CURBS, SIDEWALKS, LANDSCAPED	//.	PROPER CONSTRUCT WATERCOURSE OR W AND SEDIMENT CONT CONTAINED HEREIN. CONSERVATION DISTR
33.	AREAS, SWALE, PAVEMENT MARKINGS, OR SIGNAGE DISTURBED DURING DEMOLITION AND/OR CONSTRUCTION TO THEIR ORIGINAL CONDITION OR BETTER, AS APPROVED BY THE CIVIL ENGINEER, AND TO THE SATISFACTION OF THE OWNER AND COUNTY. EXISTING BOUNDARY AND TOPOGRAPHY IS BASED ON DRAWING TITLED "ATLA/NSPS LAND TITLE SURVEY" SCALE 1"=40', DATED 2021/02/18, BY DUFOUR SURVEYING LLC.	78.	ALL SITE WORK, MAT THE SPECIFICATIONS THE STATE DEPARTM MANUAL. ALL FILL M. PROJECT GEOTECHNIC
34. 75	ALTERNATIVE METHODS AND PRODUCTS OTHER THAN THOSE SPECIFIED MAY BE USED IF REVIEWED AND APPROVED BY THE OWNER, CIVIL ENGINEER, AND APPROPRIATE REGULATORY AGENCY PRIOR TO INSTALLATION DURING THE BIDDING PROCESS.	79.	QUALIFIED PROFESSIO THE MAXIMUM DRY D ALL DISTURBANCE IN
35. 36.	TRAFFIC PROTECTION NECESSARY FOR THE WORK. THE OWNER SHALL POST CTDOT ENCROACHMENT PERMIT BOND.	80.	BETTER, TO THE SAT
37.	AMOUNT OF THE EROSION CONTROL BOND WILL BE DETERMINED BY THE AUTHORITY HAVING JURISDICTION. NO PART OF THE PROJECT PARCEL IS LOCATED WITHIN ANY FEMA DESIGNATED FLOOD HAZARD AREAS.	81.	The utility plan de connections. Site utility or pipe con
38. 39.	THERE ARE NO WETLANDS LOCATED ON THE SITE AS INDICATED BY INLAND WETLANDS PERMIT #2017-00 AND J.R. RUSSO & ASSOCIATES MAPPING AND VISUAL OBSERVATIONS. 12" SWSB (STOP BAR) AND 4" SYDL AND SWL PAVEMENT MARKINGS LOCATED IN DRIVEWAYS AND IN STATE HIGHWAY SHALL BE EPOXY RESIN TYPE	82.	THE CONTRACTOR SH EXCAVATION. TEST P EXISTING UTILITIES, A CIVIL ENGINEER IN TH
40.	ACCORDING TO CONNDOT SPECIFICATIONS. FIRE LANES SHALL BE ESTABLISHED AND PROPERLY DESIGNATED IN ACCORDANCE WITH THE REQUIREMENTS OF THE FIRE DISTRICT FIRE MARSHAL.	83.	AND UTILITIES SO TH UTILITY CONNECTION
41. 42.	THE CONTRACTOR SHALL REMOVE CONFLICTING PAVEMENT MARKINGS IN THE ROADWAY BY METHOD APPROVED BY THE AUTHORITY HAVING JURISDICTION OR DOT AS APPLICABLE FOR THE LOCATION OF THE WORK. ALL ADA DESIGNATED PARKING STALLS, ACCESS AISLES AND PEDESTRIAN WALKWAYS SHALL CONFORM TO THE CURRENT VERSION OF THE AMERICANS	84. 85	THE CONTRACTOR SH METHODS ARE MET.
43.	WITH DISABILITIES ACT STANDARDS FOR ACCESSIBLE DESIGN AND ANSI STANDARDS AND AS MAY BE SUPERCEDED BY THE STATE BUILDING CODE. CONSTRUCTION OCCURRING ON THIS SITE SHALL COMPLY WITH NFPA 241 STANDARD FOR SAFEGUARDING CONSTRUCTION, ALTERATION AND DEMOLITION OPERATIONS, AND CHAPTER 16 OF NFPA 1 UNIFORM FIRE CODE.	00.	THE CONTRACTOR SH THE CONTRACTOR SH DISCONNECTIONS, REI GENERAL CONDITIONS
44.	ALL BUILDINGS, INCLUDING FOUNDATION WALLS AND FOOTINGS AND BASEMENT SLABS INDICATED ON THE DEMOLITION PLAN ARE TO BE REMOVED FROM THE SITE. CONTRACTOR SHALL SECURE ANY PERMITS, PAY ALL FEES AND PERFORM CLEARING AND GRUBBING AND DEBRIS REMOVAL PRIOR TO COMMENCEMENT OF GRADING OPERATIONS.	86.	ALL EXISTING PAVEMI CONTRACTOR SHALL HAVING JURISDICTION
45.	SEDIMENT AND EROSION CONTROLS AS SHOWN ON THE SEDIMENT AND EROSION CONTROL PLAN AND/OR DEMOLITION PLAN SHALL BE INSTALLED BY THE DEMOLITION CONTRACTOR PRIOR TO START OF DEMOLITION AND CLEARING AND GRUBBING OPERATIONS.	87. 88.	ALL PIPES SHALL BE SANITARY LATERAL S PROTECTION MEASURE

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON \_\_\_\_

2221 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES.

L, BY AN APPROVED HAULER. HAULER SHALL COMPLY WITH ALL REGULATORY REQUIREMENTS.

CURE ALL PERMITS FOR HIS DEMOLITION AND DISPOSAL OF HIS DEMOLITION MATERIAL TO BE REMOVED FROM THE SITE. ST BONDS AND PAY PERMIT FEES AS REQUIRED. BUILDING DEMOLITION CONTRACTOR SHALL BE RESPONSIBLE FOR ALL BUILDING DEMOLITION DEBRIS IN AN APPROVED OFF-SITE LANDFILL.

IATERIAL, IF FOUND ON SITE, SHALL BE REMOVED BY A LICENSED HAZARDOUS MATERIAL ABATEMENT CONTRACTOR. EPARE ALL MANIFEST DOCUMENTS AS REQUIRED PRIOR TO COMMENCEMENT OF DEMOLITION.

AND PLUG, OR ARRANGE FOR THE APPROPRIATE UTILITY PROVIDER TO CUT AND PLUG ALL SERVICE PIPING AT THE N, AS REQUIRED BY THE UTILITY PROVIDER, OR AS OTHERWISE NOTED OR SHOWN ON THE CONTRACT DRAWINGS. ALL IN ON THIS PLAN. THE CONTRACTOR SHALL INVESTIGATE THE SITE PRIOR TO BIDDING TO DETERMINE THE EXTENT OF VED, CUT OR PLUGGED. THE CONTRACTOR SHALL PAY ALL UTILITY PROVIDER FEES FOR ABANDONMENTS AND REMOVALS.

DTECT ALL IRON PINS, MONUMENTS AND PROPERTY CORNERS DURING DEMOLITION AND CONSTRUCTION ACTIVITIES. ANY S, MONUMENTS, AND OR PROPERTY CORNERS, ETC. SHALL BE RESET BY A LICENSED LAND SURVEYOR AT THE EXPENSE

Shall stabilize the site and keep erosion control measures in place until the completion of his work IT OF WORK BY THE SITE CONTRACTOR, WHICHEVER OCCURS FIRST, AS REQUIRED OR DEEMED NECESSARY BY THE ESENTATIVE. THE SITE CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR THE MAINTENANCE OF EXISTING EROSION AND ND FOR INSTALLATION OF ANY NEW SEDIMENT AND EROSION CONTROLS AS PER THE SEDIMENT AND EROSION CONTROL

VIP OUT BUILDING FUEL AND WASTE OIL TANKS (IF ANY ARE ENCOUNTERED) AND REMOVE FUEL TO AN APPROVED ED WASTE OIL HANDLING CONTRACTOR IN STRICT ACCORDANCE WITH STATÉ REQUIREMENTS.

TED SOIL IS ENCOUNTERED BY THE CONTRACTOR, THE CONTRACTOR SHALL SUSPEND EXCAVATION WORK OF IMPACTED AND/OR OWNER'S ENVIRONMENTAL CONSULTANT PRIOR TO PROCEEDING WITH FURTHER WORK IN THE IMPACTED SOIL STRUCTED BY THE OWNER AND/OR OWNER'S ENVIRONMENTAL CONSULTANT.

HALL BE DISCONNECTED AND CAPPED AT MAIN IN ACCORDANCE WITH THE REQUIREMENTS OF THE POTABLE WELL IG ONSITE WATER PIPING TO BE ABANDONED TO RIGHT OF WAY LINE UNLESS OTHERWISE SHOWN ON DEMOLITION PLANS TABLE WELL AUTHORITY TO BE REMOVED TO MAIN.

SHALL BE PLUGGED WITH NON-SHRINK GROUT AT CURB LINE OR AT MAIN CONNECTION IN ACCORDANCE WITH THE REQUIREMENTS. REMOVE EXISTING LATERAL PIPING FROM SITE UNLESS OTHERWISE SHOWN ON DEMOLITION PLANS OR AS UTILITY PROVIDER.

ALL BE CAPPED AND SERVICE LINES PURGED OF RESIDUAL GAS IN ACCORDANCE WITH THE GAS UTILITY PROVIDER COORDINATED BY AND PAID FOR BY THE CONTRACTOR, REMOVE EXISTING SERVICE PIPING ON SITE, ANY PROPANE RESIDUAL GAS BY PROPANE SUPPLIER. CONTRACTOR SHALL COORDINATE THIS WORK AND PAY NECESSARY FEES. DVIDE DISCONNECT NOTIFICATION TO THE MUNICIPALITY ENGINEERING DEPARTMENT, TELECOMMUNICATIONS UTILITY

IDER, ELECTRIC UTILITY PROVIDER, SANITARY UTILITY PROVIDER, AND POTABLE WELL AUTHORITY AT LEAST THREE WEEKS

ISIBLE FOR SECURING A DEMOLITION PERMIT FROM THE MUNICIPALITY BUILDING DEPARTMENT AND MUST FURNISH THE RIAL AND PAY ALL FEES.

JNDATION HOLES AND REMOVED DRIVEWAY AREAS IN LOCATIONS NOT SUBJECT TO FURTHER EXCAVATION WITH SOIL OWNER'S GEOTECHNICAL ENGINEER AND COMPACT, FERTILIZE, SEED AND MULCH DISTURBED AREAS NOT SUBJECT TO . DEMOLISHED BUILDING FOUNDATION AREA AND BASEMENT IF PRESENT TO BE BACKFILLED WITH GRAVEL FILL OR PROJECT GEOTECHNICAL REPORT IN LIFT THICKNESS SPECIFIED IN THE GEOTECHNICAL REPORT. COMPACT TO 95% MAX. 57 AT MOISTURE CONTENT SPECIFIED IN GEOTECHNICAL REPORT AND EARTHWORK SPECIFICATION. EMPLOY WATERING

PAIR PAVEMENTS BY INSTALLING TEMPORARY AND PERMANENT PAVEMENTS IN PUBLIC RIGHTS OF WAYS AS REQUIRED BY es and the municipality and per permit requirements due to demolition and pipe removal activities.

AND REMOVE AT LUMINARE AND SIGN LOCATIONS ANY PROTRUDING CONDUITS TO 24" BELOW GRADE. THE ALL CABLE AND CONDUCTORS FROM REMAINING LIGHTING AND SIGNING CONDUITS TO BE ABANDONED. ANY REMAINING E SHALL BE RECIRCUITED OR REWIRED AS NECESSARY TO REMAIN IN OPERATION.

LL BE INITIATED BY THE CONTRACTOR UNTIL A PRE-CONSTRUCTION MEETING WITH OWNER AND THE CIVIL ENGINEER IS IR SHOULD BE AWARE OF ANY SITE INFORMATION AVAILABLE SUCH AS GEOTECHNICAL AND ENVIRONMENTAL REPORTS. VE CBYD MARK OUTS OF EXISTING UTILITIES COMPLETED PRIOR TO MEETING.

RANGE FOR AND INSTALL TEMPORARY OR PERMANENT UTILITY CONNECTIONS WHERE INDICATED ON PLAN OR AS SERVICES TO BUILDINGS OR TO SERVICES TO REMAIN. CONTRACTOR TO COORDINATE WITH UTILITY PROVIDERS FOR TY PROVIDER FEES.

COMMENCE DEMOLITION OR UTILITY DISCONNECTIONS UNTIL AUTHORIZED TO DO SO BY THE OWNER.

ITION CONTRACTOR SHALL INSTALL TEMPORARY SHEETING OR SHORING AS NECESSARY TO PROTECT EXISTING AND NEW UTILITIES DURING CONSTRUCTION AND DEMOLITION. SHEETING OR SHORING SHALL BE DESIGNED BY A PROFESSIONAL STATE AND EVIDENCE OF SUCH SUBMITTED TO THE OWNER PRIOR TO INSTALLATION. AITTED UNLESS PAID TO THE OWNER AS A CREDIT.

AND ANY EXISTING SEPTIC TANKS/ABSORPTION AREAS SHALL BE ABANDONED AND REMOVED PER THE DEEP AND

ALL REMAIN OPEN FOR NORMAL BUSINESS OPERATIONS UNTIL COMPLETION AND OCCUPATION OF THE NEW BUILDING. ESERVE EXISTING VEGETATION WHERE POSSIBLE AND/OR AS NOTED ON DRAWINGS. REFER TO SEDIMENT AND EROSION DISTURBANCE AND EROSION CONTROL NOTES.

AND STOCKPILED ON SITE FOR USE IN FINAL LANDSCAPING.

WITH REMOVAL AND REPLACEMENT OF FILL AND REMOVAL AND REPLACEMENT OF UNSUITABLE AND SOFT SUBGRADE THE GEOTECHNICAL ENGINEER. SEE GEOTECHNICAL REPORT AND EARTHWORK SPECIFICATIONS FOR FURTHER DESCRIPTION. VPACT FILL IN LIFT THICKNESS PER THE GEOTECHNICAL REPORT UNDER ALL PARKING, BUILDING, DRIVE, AND STRUCTURE MUM DRY DENSITY AS DETERMINED BY ASTM D1557 (MODIFIED PROCTOR TEST), OR AS REQUIRED BY THE GEOTECHNICAL

DED, IF DETERMINED NECESSARY IN THE FIELD BY THE OWNER/GEOTECHNICAL ENGINEER, AFTER SUBGRADE IS ROUGH

PHYSICALLY MARKED IN THE FIELD AND APPROVED BY THE MUNICIPALITY'S AGENT PRIOR TO THE START OF WORK ON

CEDURES SHALL BE FOLLOWED ON ALL IMPROVEMENTS WITHIN THIS PARCEL SO AS TO PREVENT THE SILTING OF ANY IN ACCORDANCE WITH THE REGULATIONS OF THE CT DEEP AND THE 2002 CONNECTICUT GUIDELINES FOR SOIL EROSION TEST EDITION. IN ADDITION, THE CONTRACTOR SHALL STRICTLY ADHERE TO THE SEDIMENT AND EROSION CONTROL PLAN ITRACTOR SHALL BE RESPONSIBLE TO POST ALL BONDS AS REQUIRED BY THE LOCAL MUNICIPALITIES, OR SOIL CH WOULD GUARANTEE THE PROPER IMPLEMENTATION OF THE PLAN.

OF CONSTRUCTION, AND CONSTRUCTION METHODS FOR EARTHWORK AND STORM DRAINAGE WORK SHALL CONFORM TO TAILS AND APPLICABLE SECTIONS OF THE PROJECT SPECIFICATIONS MANUAL. OTHERWISE THIS WORK SHALL CONFORM TO TRANSPORTATION SPECIFICATIONS AND PROJECT GEOTECHNICAL REPORT IF THERE IS NO PROJECT SPECIFICATIONS UNDER STRUCTURES AND PAVED AREAS SHALL BE PER THE ABOVE STATED APPLICABLE SPECIFICATIONS, AND/OR ORT, AND SHALL BE PLACED IN ACCORDANCE WITH THE APPLICABLE SPECIFICATIONS UNDER THE SUPERVISION OF A GINEER. MATERIAL SHALL BE COMPACTED IN LIFT THICKNESSES PER THE PROJECT GEOTECHNICAL REPORT TO 95% OF AS DETERMINED BY ASTM D 1557 AT MOISTURE CONTENT INDICATED IN PROJECT GEOTECHNICAL REPORT.

TO MUNICIPAL AND STATE PROPERTY DUE TO CONSTRUCTION SHALL BE RESTORED TO ITS PREVIOUS CONDITION OR ON OF THE MUNICIPALITY AND STATE AS APPLICABLE FOR THE LOCATION OF THE WORK.

DOT RIGHT OF WAY SHALL COMPLY WITH ALL DEPARTMENT OF TRANSPORTATION STANDARDS AND SPECIFICATIONS.

ITE INSTALLED PIPES UP TO 5' FROM THE BUILDING FACE. REFER TO DRAWINGS BY ARCHITECT FOR BUILDING CTOR SHALL SUPPLY AND INSTALL PIPE ADAPTERS AS NECESSARY AT BUILDING CONNECTION POINT OR AT EXISTING

IT THE SITE AND VERIFY THE ELEVATION AND LOCATION OF ALL UTILITIES BY VARIOUS MEANS PRIOR TO BEGINNING ANY LL BE DUG AT ALL LOCATIONS WHERE PROPOSED SANITARY SEWERS AND WHERE PROPOSED STORM PIPING WILL CROSS HORIZONTAL AND VERTICAL LOCATIONS OF THE UTILITIES SHALL BE DETERMINED. THE CONTRACTOR SHALL CONTACT THE T OF ANY DISCOVERED OR UNFORESEEN CONFLICTS BETWEEN EXISTING AND PROPOSED SANITARY SEWERS, STORM PIPING APPROPRIATE MODIFICATION MAY BE MADE.

AS REFLECTED ON THE PLAN MAY CHANGE SUBJECT TO UTILITY PROVIDER AND GOVERNING AUTHORITY STAFF REVIEW. SURE THAT ALL UTILITY PROVIDERS AND GOVERNING AUTHORITY STANDARDS FOR MATERIALS AND CONSTRUCTION ITRACTOR SHALL PERFORM PROPER COORDINATION WITH THE RESPECTIVE UTILITY PROVIDER.

RANGE FOR AND COORDINATE WITH THE RESPECTIVE UTILITY PROVIDERS FOR SERVICE INSTALLATIONS AND CONNECTIONS. ORDINATE WORK TO BE PERFORMED BY THE VARIOUS UTILITY PROVIDERS AND SHALL PAY ALL FEES FOR CONNECTIONS, NS, INSPECTIONS, AND DEMOLITION UNLESS OTHERWISE STATED IN THE PROJECT SPECIFICATIONS MANUAL AND/OR CONTRACT.

TRE UTILITY PIPING IS TO BE INSTALLED SHALL BE SAW CUT. AFTER UTILITY INSTALLATION IS COMPLETED, THE TEMPORARY AND/OR PERMANENT PAVEMENT REPAIR AS DETAILED ON THE DRAWINGS OR AS REQUIRED BY THE OWNER

N STRAIGHT ALIGNMENTS AND EVEN GRADES USING A PIPE LASER OR OTHER ACCURATE METHOD.

AINTAIN (10' MIN. HORIZONTAL 1.5' VERTICAL MIN.) SEPARATION DISTANCE FROM WATER LINES, OR ADDITIONAL BE REQUIRED WHERE PERMITTED, WHICH SHALL INCLUDE CONCRETE ENCASEMENT OF PIPING UNLESS OTHERWISE

	DEFINITIONS	Jre Jg syinç
89. RELOCATION OF UTILITY PROVIDER FACILITIES SHALL BE DONE IN ACCORDANCE WITH THE REQUIREMENTS OF THE UTILITY PROVIDER.	MUNICIPALITY SHALL MEAN <u>TOWN OF BOLTON</u>	ect. >erit >urve
90. THE CONTRACTOR SHALL COMPACT THE PIPE BACKFILL IN 8" LIFTS ACCORDING TO THE PIPE BEDDING DETAILS. TRENCH BOTTOM SHALL BE STABLE	COUNTY SHALL MEAN TOLLAND COUNTY	chite igine ind S
91. CONTRACTOR TO PROVIDE STEEL SLEEVES AND ANNULAR SPACE SAND FILL FOR UTILITY PIPE AND CONDUIT CONNECTIONS UNDER FOOTINGS.	STATE SHALL MEAN <u>CONNECTICUT</u>	Arr.
92. BUILDING UTILITY PENETRATIONS AND LOCATIONS ARE SHOWN FOR THE CONTRACTOR'S INFORMATION AND SHALL BE VERIFIED WITH THE BUILDING	POTABLE WELL AUTHORITY SHALL MEAN <u>EASTERN HIGHLANDS HEALTH DISTRICT</u>	
MEP, STRUCTURAL, AND ARCHITECTURAL DRAWINGS AND WITH THE OWNER'S CONSTRUCTION MANAGER. 93. ALL UTILITY CONSTRUCTION IS SUBJECT TO INSPECTION FOR APPROVAL PRIOR TO BACKFILLING, IN ACCORDANCE WITH THE APPROPRIATE UTILITY	GAS UTILITY PROVIDER SHALL MEAN <u>PROPANE TANK PROVIDER</u>	
PROVIDER REQUIREMENTS.	TELECOMMUNICATIONS UTILITY PROVIDER SHALL MEAN FRONTIER COMMUNICATIONS OF CONNECTICUT	
94. A ONE-FOOT MINIMUM VERTICAL CLEARANCE BETWEEN WATER, GAS, ELECTRICAL, AND TELEPHONE LINES AND STORM PIPING SHALL BE PROVIDED. A SIX-INCH MINIMUM CLEARANCE SHALL BE MAINTAINED BETWEEN STORM PIPING AND SANITARY SEWER WITH A CONCRETE ENCASEMENT. AN 18-INCH TO 6-INCH VERTICAL CLEARANCE BETWEEN SANITARY SEWER PIPING AND STORM PIPING SHALL REQUIRE CONCRETE ENCASEMENT OF THE PROPOSED PIPING.	ELECTRIC UTILITY PROVIDER SHALL MEAN EVERSOURCE ENERGY - ELECTRIC DISTRIBUTION	100 Constitution Plaz
95. GRAVITY SANITARY SEWER PIPING AND PRESSURIZED WATERLINES SHALL BE LOCATED IN SEPARATE TRENCHES AT LEAST 10 FEET APART WHENEVER POSSIBLE. WHEN INSTALLED IN THE SAME TRENCH, THE WATER PIPE SHALL BE LAID ON A TRENCH BENCH AT LEAST 18 INCHES ABOVE THE TOP OF THE SANITARY SEWER PIPE AND AT LEAST 12 INCHES (PREFERABLY 18 INCHES) FROM THE SIDE OF THE SANITARY SEWER PIPE TRENCH.		10th Floor Hartford, CT 06103 (860) 249-2200 (860) 249-2400 Fax
96. SITE CONTRACTOR SHALL PROVIDE ALL BENDS, FITTINGS, ADAPTERS, ETC., AS REQUIRED FOR PIPE CONNECTIONS TO BUILDING STUB OUTS, INCLUDING ROOF/FOOTING DRAIN CONNECTIONS TO ROOF LEADERS AND TO STORM DRAINAGE SYSTEM.		
97. MANHOLE RIMS AND CATCH BASIN GRATES SHALL BE SET TO ELEVATIONS SHOWN. SET ALL EXISTING MANHOLE RIMS AND VALVE COVERS TO BE RAISED OR LOWERED FLUSH WITH FINAL GRADE AS NECESSARY.		UNING FOONAGE
98. SITE CONTRACTOR SHALL COORDINATE INSTALLATION OF CONDUIT AND CABLES FOR SITE LIGHTING WITH THE BUILDING ELECTRICAL CONTRACTOR.		A CALLER AND
99. CONTRACTOR SHALL COORDINATE INSTALLATION FOR ELECTRICAL SERVICES TO PYLON SIGNS AND SITE LIGHTING WITH THE BUILDING ELECTRICAL CONTRACTOR.		
100. THE CONTRACTOR SHALL ARRANGE AND COORDINATE WITH UTILITY PROVIDERS FOR WORK TO BE PERFORMED BY UTILITY PROVIDERS. THE CONTRACTOR SHALL PAY ALL UTILITY FEES UNLESS OTHERWISE STATED IN THE PROJECT SPECIFICATION MANUAL AND GENERAL CONDITIONS, AND REPAIR PAVEMENTS AS NECESSARY.		BALLENSED.
101. ELECTRIC, AND TELECOMMUNICATIONS SERVICES SHALL BE INSTALLED UNDERGROUND FROM THE SERVICE POLE INDICATED ON THE SITE UTILITIES PLAN. THE CONTRACTOR SHALL PROVIDE AND INSTALL AND BACKFILL (2) 4" PVC CONDUITS FOR TELECOMMUNICATIONS SERVICE, (2) 4" PVC CONDUITS FOR ELECTRIC SERVICE PRIMARY, PVC CONDUITS FOR ELECTRICAL SECONDARY PER BUILDING ELECTRICAL PLANS, (SCHEDULE 80 UNDER PAVEMENT, SCHEDULE 40 IN NON PAVEMENT AREAS). SERVICES MAY BE INSTALLED IN A COMMON TRENCH WITH 12" CLEAR SPACE BETWEEN. MINIMUM COVER IS 36" ON ELECTRIC CONDUITS, AND 24" ON TELECOMMUNICATIONS CONDUITS. SERVICES SHALL BE MARKED WITH MAGNETIC LOCATOR TAPE AND SHALL BE BEDDED, INSTALLED, AND BACKFILLED IN ACCORDANCE WITH ELECTRIC UTILITY PROVIDER, AND TELECOMMUNICATIONS COMPANY STANDARDS. GALVANIZED STEEL ELECTRICAL CONDUIT SHALL BE USED AT POLE AND TRANSFORMER LOCATIONS. INSTALL HANDHOLES AS REQUIRED TO FACILITATE INSTALLATION AND AS REQUIRED BY UTILITY PROVIDER. INSTALL TRAFFIC LOAD QUALIFIED HANDHOLES IN VEHICULAR AREAS. INSTALL CONCRETE ENCASEMENT ON PRIMARY ELECTRIC CONDUITS IF REQUIRED BY ELECTRIC UTILITY PROVIDER.		
102. ALL WATER LINES TO HAVE A MINIMUM COVER OF 3'-6". ALL LINES SHALL BE BEDDED IN 6" SAND AND INITIALLY BACKFILLED WITH 12" SAND.		
103. ALL WATER MAINS, WATER SERVICES AND SANITARY SEWER LATERALS SHALL CONFORM TO THE APPLICABLE POTABLE WELL AUTHORITY SPECIFICATIONS, AND TO THE APPLICABLE SANITARY SEWER PROVIDER SPECIFICATIONS, AS WELL AS TO OTHER APPLICABLE INDUSTRY CODES (AWWA), CTDPH, AND PROJECT SPECIFICATIONS FOR POTABLE WATER SYSTEMS, AND FOR SANITARY SEWER SYSTEMS.		L Z
104. THE CONTRACTOR SHALL MAINTAIN ALL FLOWS AND UTILITY CONNECTIONS TO EXISTING BUILDINGS WITHOUT INTERRUPTION UNLESS/UNTIL AUTHORIZED TO DISCONNECT BY THE OWNERS, THE CIVIL ENGINEER, UTILITY PROVIDERS AND GOVERNING AUTHORITIES.		
105. THE CONTRACTOR MAY SUBSTITUTE MASONRY STRUCTURES FOR PRECAST STRUCTURES IF APPROVED BY THE CIVIL ENGINEER AND ALLOWED BY THE GOVERNING AUTHORITY ENGINEER OR OTHER GOVERNING AUTHORITY.		I I I
106. PIPING SHALL BE LAID FROM DOWNGRADIENT END OF PIPE RUN IN AN UPGRADIENT DIRECTION WITH BELL END FACING UPGRADE IN THE DIRECTION OF PIPE LAYING.		
107. ALL RCP SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-76; ALL RCP SHALL BE CLASS IV UNLESS OTHERWISE SHOWN. JOINTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-443.		
108. MANHOLE SECTIONS AND CONSTRUCTION SHALL CONFORM TO ASTM C-478.		
109. HIGH DENSITY POLYETHYLENE (HDPE) STORM SEWER 12" OR GREATER IN DIAMETER SHALL BE HI-Q SURE-LOK 10.8 PIPE AS MANUFACTURED BY HANCOR INC. OR APPROVED EQUAL. HDPE PIPE SHALL HAVE SMOOTH INTERIOR AND CORRUGATED EXTERIOR AND SHALL MEET THE REQUIREMENTS OF AASHTO M294, TYPE S. PIPE SECTIONS SHALL BE JOINED WITH BELL-AND-SPIGOT JOINT MEETING THE REQUIREMENTS OF AASHTO M294. THE BELL SHALL BE AN INTEGRAL PART OF THE PIPE AND PROVIDE A MINIMUM PULL-APART STRENGTH OF 400 POUNDS. THE JOINT SHALL BE WATERTIGHT ACCORDING TO THE REQUIREMENTS OF ASTM D3212. GASKETS SHALL BE MADE OF POLYISOPRENE MEETING THE REQUIREMENTS OF ASTM F477. ALTERNATIVE HDPE PIPE MAY BE USED IF APPROVED BY THE ENGINEER AND OWNER'S CONSTRUCTION MANAGER PRIOR TO ORDERING.		ETAIL I DSTON T CONP
110. HIGH DENSITY POLYETHYLENE (HDPE) STORM SEWER LESS THAN 12" IN DIAMETER SHALL BE HI-Q PIPE AS MANUFACTURED BY HANCOR INC. OR APPROVED EQUAL. HDPE PIPE SHALL HAVE SMOOTH INTERIOR AND CORRUGATED EXTERIOR AND SHALL MEET THE REQUIREMENTS OF AASHTO 252, TYPE S. PIPE SECTIONS SHALL BE JOINED WITH COUPLING BANDS OR EXTERNAL SNAP COUPLERS COVERING AT LEAST 2 FULL CORRUGATIONS ON EACH END OF THE PIPE. SILT-TIGHT (GASKET) CONNECTIONS SHALL INCORPORATE A CLOSED SYNTHETIC EXPANDED RUBBER GASKET. MEETING THE REQUIREMENTS OF AASHTO D1056 GRADE 2A2. GASKETS SHALL BE INSTALLED ON THE CONNECTION BY THE PIPE MANUFACTURER. ALTERNATIVE HDPE PIPE MAY BE USED IF APPROVED BY THE ENGINEER AND OWNER'S CONSTRUCTION MANAGER PRIOR TO ORDERING.		SED RI 1100 B(
111. COPPER PIPE SHALL BE TYPE K TUBING WITH COMPRESSION FITTINGS.		Ô
112. GAS PIPE MATERIAL SHALL BE PER GAS COMPANY REQUIREMENTS.		<del> </del>
113. POLYVINYL CHLORIDE PIPE (PVCP) FOR SANITARY PIPING SHALL HAVE BUILT-IN RUBBER GASKET JOINTS. PVCP SHALL CONFORM TO ASTM D3034 (SDR35) WITH COMPRESSION JOINTS AND MOLDED FITTINGS. PVCP SHALL BE INSTALLED IN ACCORDANCE WITH THE DETAILS, ASTM D2321 AND MANUFACTURER'S RECOMMENDED PROCEDURE.		PRO
114. PVC WATER MAIN PIPING SHALL CONFORM TO AWWA C900.		

Desc. REVISED PER TOWN COMMENTS	
REVISIONS No. Date 1. 05/05/2021	
 Designed Drawn Reviewed Scale Project No. Date CAD File: GN200203201	S.E.L. S.E.L. K.M.M. NONE 2002032 04/02/2021
Title GENERAL NOTES Sheet No.	
GN	-1

### FOR PERMITTING PURPOSES ONLY NOT RELEASED FOR CONSTRUCTION

# **DEMOLITION LEGEND**







PROPERTY LINE LIMIT OF DISTURBANCE AND SITEWORK CONTRACT LIMIT LINE SAWCUT LINE

REMOVE AND DISPOSE OF CURB, FENCE, ETC. PROTECT EXISTING UTILITY LINE

LIMIT OF TREE AND VEGETATION CLEARING REMOVE AND DISPOSE OF SIGN, HYDRANT, FIXTURE, ETC.

REMOVE AND DISPOSE OF EXISTING BITUMINOUS CONCRETE PAVEMENT STRUCTURE

REMOVE AND DISPOSE OF EXISTING TREE AND STUMP

PROTECT EXISTING TREE TO REMAIN

NCROACHMENT	
EXEPTION #3	

N/F

N/F DREW & HARRY .182- PG.1074 7 NORTH RD

N/F



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DATE APPROVED	DATE OF EXPIRATION	
		CHAIRMAN

21 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES.

# **ZONING INFORMATION**

LOCATION: BOLTON, TOLLAND COUNTY, CONNECTICUT

ZONE: RURAL MIXED USE ZONE (RMUZ)

USE: RETAIL (PERMITTED BY SPECIAL PERMIT)

ITEM #	ITEM	REQUIREMENTS	PROPOSED	VARIANCE
1	MINIMUM LOT AREA	80,000 S.F.	80,707 S.F. (1.85 AC.)	NO
2	MINIMUM LOT WIDTH	NONE REQUIRED	308 FEET	NO
3	MINIMUM LOT FRONTAGE	150 FEET	260.4 FEET	NO
4	MINIMUM FRONT SETBACK	NONE REQUIRED	71.9 FEET	NO
5	MINIMUM SIDE SETBACK	25 FEET (50 FEET) [1]	72.8 FEET	NO
6	MINIMUM REAR SETBACK	25 FEET [1]	51.4 FEET	NO
7	MAXIMUM BUILDING HEIGHT	35 FEET/2.5 STORIES	25.6 FEET	NO
8	MAXIMUM BUILDING COVERAGE	25 PERCENT	13.2 PERCENT	NO
9	MAXIMUM IMPERVIOUS COVERAGE	50 PERCENT	39.9 PERCENT	NO

LINE

[1] MINIMUM SIDE AND REAR SETBACKS - 50 FEET WHEN ABUTTING A RESIDENTIAL DISTRICT

# PARKING INFORMATION

					AIV ANDILLW
ITEM #	ITEM	REQUIREMENTS	PROPOSED	VARIANCE	/OL.53- PG.792
1	BUILDING SIZE	600 S.F.	10,640 S.F.	NO	
2	PARKING REQUIRED	RETAIL: MINIMUM - 2 SPACES PER 1,000 S.F. OF GFA (10,640 S.F.) MINIMUM REQUIRED = 22 SPACES MAXIMUM - 5 SPACES PER 1,000 S.F. OF GFA (10,640 S.F.) MAXIMUM ALLOWED = 54 SPACES	33 SPACES	NO	PROVIDE AND INSTALL MC PROVIDE AND INSTALL MC CONCRETE CURB AND SIDEWAL 8' LANDSCAPE AROUND BUILT
3	MINIMUM HANDICAPPED PARKING SPACES REQUIRED	2 SPACES	2 SPACES	NO	PROVIDE AND INS
4	MINIMUM PARKING DIMENSIONS	9 FEET X 18 FEET	9 FEET X 20 FEET	NO	
5	MINIMUM LOADING DIMENSIONS	10 FEET X 25 FEET X 14 FEET	33 FEET X 71 FEET X > 14 FEET	NO	
6	MINIMUM AISLE WIDTH	22 FEET – 2–WAY 11 FEET – 1–WAY	30 FEET - 2-WAY	NO	END MONOLITHIC CONCR AND SIDEWALK, BEGIN BI
7	MINIMUM FRONT SETBACK	50 FEET [2]	50.5 FEET	NO	CONCRE END BITUMINOUS CONCRE
8	MINIMUM SIDE SETBACK	NONE REQUIRED [2]	77.1 FEET	NO	BEGIN MONOLITHIC CONCR AND
9	MINIMUM REAR SETBACK	NONE REQUIRED [2]	5.3 FEET	NO	REW PROVIDE AND INSTALL CONCRETE SIDEWALK ST
10	BICYCLE PARKING REQUIRED	1 BICYLE PARKING SPACE PER 25 PARKING STALLS (2 REQUIRED)	2 BICYCLE PARKING SPACES	NO	PROVIDE AND INSTALL 182— PG.107connecticut sy Accessibility (ty

[2] 10 FEET LANDSCAPED BUFFER STRIP REQUIRED WHERE ABUTTING A RESIDENCE DISTRICT

# SITE PLAN LEGEND



PROPERTY LINE LIMIT OF DISTURBANCE AND SITEWORK CONTRACT LIMIT LINE SAWCUT LINE

PROVIDE AND INSTALL CONCRETE PAVEMENT STRUCTURE, REINFORCED CONCRETE SIDEWALK, OR MONOLITHIC CONCRETE CURB AND SIDEWALK

PROVIDE AND INSTALL FULL DEPTH HEAVY DUTY BITUMINOUS CONCRETE PAVEMENT STRUCTURE

PROVIDE AND INSTALL FULL DEPTH STANDARD DUTY BITUMINOUS CONCRETE PAVEMENT STRUCTURE

PROVIDE AND INSTALL SIGN

# SIGN LEGEND

•

SIGN NO.	C-DOT NO.	LEGEND	
A	31–0552Z	STOP 30"	
В	31–0629	WILKING WIL EF ROD PARISON HAADDCAPPED PARKING FAIR HINT RELAND	
с	31–0648	VAN ACCESSIBLE	
NOTE: 1. HANDICAPPED SIGNS TO BE INSTALLED IN PIPE BOLLARDS (SEE DETAIL). ALL HANDICAP SIGNAGE TO CONFORM TO LATEST BUILDING CODE.			

SIGNS INSTALLED IN THE STATE RIGHT-OF-WAY MUST BE INSTALLED IN ACCORDANCE WITH THE DEPARTMENT'S TYPICAL DETAIL SHEETS (I.E. HEIGHT, BREAKAWAY POSTS, ETC.)

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED DATE OF EXPIRATION

\_CHAIRMAN

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON

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# GRADING AND DRAINAGE LEGEND

PROPERTY LINE

SAWCUT LINE

		LOD		 _
	_		—	 _

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×100.00

STORM LINE MANHOLE CATCH BASIN PROPOSED CONTOUR LINE

PROPOSED SPOT GRADE



BOTTOM OF WALL TOP OF WALL MEET EXISTING CONDITION BW TW MEX

LIMIT OF DISTURBANCE AND SITEWORK CONTRACK LIMIT LINE

ENCROACHMENT	S
EXEPTION #3	

N/F		
ARLES	&	MARIE
DL.41- PG	.95	
5 NORTH	RD	

N/F

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DATE APPROVED	DATE OF EXPIRATION	
		CHAIRMAN

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# SITE UTILITIES LEGEND

	PROPERTY LI
LOD	LIMIT OF DIS CONTRACT LI
	SAWCUT LINE
—— Е —— Е ——	ELECTRIC LIN
G G	GAS LINE
——— w ——— w ———	WATER LINE
—— s —— s ——	SANITARY SE
SFM	SANITARY SE
— T T	TELECOMMUN
ETC	ELECTRIC AN
	STORM LINE

PROPERTY LINE LIMIT OF DISTURBANCE AND SITEWORK CONTRACT LIMIT LINE SAWCUT LINE ELECTRIC LINE GAS LINE WATER LINE SANITARY SEWER LINE SANITARY SEWER FORCE MAIN TELECOMMUNICATIONS LINE

ELECTRIC AND TELECOMMUNICATIONS LINE





N/FARLES & MARIE DL.41- PG.95 5 NORTH UNDERGROUND PROPANE TANK AND ASSOCIATED BOLLARDS,

PROVIDE AND INSTALL PVC SCH 80 CONDUITS-FOR ELECTRICAL SECONDARY SERVICE TO BUILDING, COORDINATE NUMBER, TYPE, AND SIZE WITH BUILDING ELECTRICAL PLANS

N/F PROVIDE AND INSTALL (2) 4" SCH 80-PVC CONDUITS FOR TELECOMMUNICATION SERVICE TO BUILDING WITH PULL AK VOL.53- PG.792

PROVIDE AND INSTALL WELL, COORDINATE

COPPER WATER LINE FOR DOMESTIC WATER SERVICE CONNECTION TO

N/F PROVIDE AND INSTALL WATER METER, COORDINATE WITH BOLTON LAKES REGIONAL HEALTH AUTHORITY )RE .182- PG.1074

FOR ELECTRICAL CONNECTION AND

N/F LORETTA GRACE /OL.147- PG.102

184 BOSTE ONE DUPLEX DH152-93 PUMP MODIFIED TO ACCEPT 6" PIPE,

PROVIDE AND INSTALL (1) 2" PVC-SCH40 CONDUIT WITH PULL STRING FOR ELECTRIC SIGN CIRCUIT

	PROVIDE AND UNI-LATERAL S LATERAL	INSTALL E/ON STAINLESS STEP VALVE AND K
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61 A	613	
CON	INECT TO EXISTING	LOW PRESSUR

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THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON \_

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# **EROSION CONTROL LEGEND**



PROPERTY LINE LIMIT OF DISTURBANCE AND SITEWORK CONTRACT LIMIT LINE SAWCUT LINE SILT FENCE BARRIER

SILT SACK INLET PROTECTION

CONCRETE WASH PIT

TEMPORARY MATERIAL STOCKPILE

EROSION CONTROL BLANKET

CONSTRUCTION ENTRANCE

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EXEPTION #3	

N/F ARLES & MARIE )L.41- PG.95 5 NORTH RD

N/F AR ANDREW VOL.53- PG.792

11 NORTH RD

N/F DREW & HARRY .182- PG.1074 7 NORTH RD

N/F LORETTA GRACE /OL.147- PG.102 184 BOSTON TPKE

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### SEDIMENT AND EROSION CONTROL NOTES

SEDIMENT & EROSION CONTROL NARRATIVE THE SEDIMENT AND EROSION CONTROL PLAN WAS DEVELOPED TO PROTECT THE EXISTING ROADWAY AND SYSTEMS. ADJACENT PROPERTIES. AND ANY ADJACENT WETLAND AREA AND ANY ADJACENT WATER COU LADEN SURFACE RUNOFF AND EROSION. A CONSTRUCTION SEQUENCE IS PROVIDED TO PROVIDE SURFAC CONTROLS PRIOR TO THE BEGINNING OF PROJECT DEMOLITION AND/OR CONSTRUCTION. CONSTRUCTION SCHEDULE

THE ANTICIPATED STARTING DATE FOR CONSTRUCTION IS SPRING 2021 WITH COMPLETION ANTICIPATED F APPROPRIATE SEDIMENT AND EROSION CONTROL MEASURES AS DESCRIBED HEREIN SHALL BE INSTALLED PRIOR TO THE COMMENCEMENT OF ALL DEMOLITION OR CONSTRUCTION ACTIVITY. SCHEDULE WORK TO MII TIME THAT BARE SOIL WILL BE EXPOSED.

CONTINGENCY EROSION PLAN THE CONTRACTOR SHALL INSTALL ALL SPECIFIED SEDIMENT AND EROSION CONTROL MEASURES AND WILL MAINTAIN THEM IN THEIR INTENDED FUNCTIONING CONDITION. THE AGENTS OF THE MUNICIPALITY OR INLA COMMISSION AND/OR CIVIL ENGINEER SHALL HAVE THE AUTHORITY TO REQUIRE SUPPLEMENTAL MAINTEN MEASURES IF FIELD CONDITIONS ARE ENCOUNTERED BEYOND WHAT WOULD NORMALLY BE ANTICIPATED.

CONSTRUCTION SEQUENCE THE FOLLOWING CONSTRUCTION SEQUENCE IS RECOMMENDED:

- 1. CONTACT MUNICIPALITY OR INLAND WETLANDS COMMISSION AGENT AT LEAST FORTY-EIGHT (48) HOU COMMENCEMENT OF ANY DEMOLITION, CONSTRUCTION OR REGULATED ACTIVITY ON THIS PROJECT.
- 2. CLEARING LIMITS SHALL BE PHYSICALLY MARKED IN THE FIELD AND APPROVED BY THE MUNICIPALITY COMMISSION AGENT PRIOR TO THE START OF WORK ON THE SITE. INSTALL TREE PROTECTION AND P
- 3. CONSTRUCT STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS AT CONSTRUCTION ENTRANCES FILTER FABRIC AROUND GRATES OF CATCH BASINS OR INSTALL SILT SACKS ON CATCH BASIN INLETS INSTALL SILT FENCE AND OTHER EROSION CONTROL DEVICES INDICATED ON THESE PLANS AT PERIME SITE DISTURBANCE AND INSTALL ALL EROSION CONTROL MEASURES AND TREE PROTECTION INDICATE INSTALL SEDIMENT BASINS AND SEDIMENT TRAPS IF REQUIRED AT LOW AREAS OF SITE OR AS ORDER OR AS SHOWN ON THESE PLANS.
- 4. CLEAR AND GRUB SITE. STOCKPILE CHIPS. STOCKPILE TOPSOIL. INSTALL SEDIMENT AND EROSION CO STOCKPILES.
- 5. ANY BUILDING AND SITE DEMOLITION AND REMOVAL. PAVEMENT REMOVAL.
- 6. INSTALL SILT FENCE, CONSTRUCT ANY DIVERSION SWALES AND SEDIMENT BASINS AND SEDIMENT TRA INSTALLATION OF STORM DRAINAGE SYSTEM.
- 7. COMMENCE EARTHWORK. INSTALL ADDITIONAL SEDIMENT AND EROSION CONTROLS AS WORK PROGRES STORM DRAINAGE SYSTEM CONSTRUCTION, TOPSOIL AND SEED SLOPES WHICH HAVE ACHIEVED FINAL
- 8. CONSTRUCTION STAKING OF ALL BUILDING CORNERS, UTILITIES, ACCESS DRIVES, AND PARKING AREAS
- 9. ROUGH GRADING AND FILLING OF SUBGRADES AND SLOPES.
- 10. IMMEDIATELY UPON DISCOVERING UNFORESEEN CIRCUMSTANCES POSING THE POTENTIAL FOR ACCELEF SEDIMENT POLLUTION, THE OPERATOR SHALL IMPLEMENT APPROPRIATE BEST MANAGEMENT PRACTICES POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION.
- 11. BEFORE DISPOSING OF SOIL OR RECEIVING BORROW FOR THE SITE, THE CONTRACTOR MUST PROVIDE SPOIL OR BORROW AREA HAS A SEDIMENT AND EROSION CONTROL PLAN APPROVED BY THE MUNICIF WETLANDS COMMISSION AND WHICH IS BEING IMPLEMENTED AND MAINTAINED. THE CONTRACTOR SH MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION IN WRITING OF ALL RECEIVING SPOIL AND BOR THEY HAVE BEEN IDENTIFIED.
- 12. CONTINUE INSTALLATION OF STORM DRAINAGE AS SUBGRADE ELEVATIONS ARE ACHIEVED.
- 13. BUILDING FOUNDATION SUBGRADE AND PAD SUBGRADE PREPARATION.
- 14. BUILDING FOUNDATION CONSTRUCTION. BEGIN BUILDING SUPERSTRUCTURE
- 15. THROUGHOUT CONSTRUCTION SEQUENCE, REMOVE SEDIMENT FROM BEHIND ANY SILT FENCES, HAY BA EROSION CONTROL DEVICES, AND FROM SEDIMENT BASINS AND SEDIMENT TRAPS AS REQUIRED. REMO PERIODIC BASIS (EVERY SIGNIFICANT RAINFALL OF 0.25 INCH OR GREATER). INSPECTION OF SEDIMEN CONTROL MEASURES SHALL BE ON A WEEKLY BASIS AND AFTER EACH RAINFALL OF 0.25 INCHES OF COLLECTED SHALL BE DEPOSITED AND SPREAD EVENLY UPLAND ON SLOPES DURING CONSTRUCTION.
- 16. INSTALL SANITARY LATERAL AND UTILITIES. COMPLETE STORM DRAINAGE SYSTEM.
- 17. INSTALL SITE LIGHTING AND TRASH ENCLOSURE.
- 18. COMPLETE GRADING TO SUBGRADES AND CONSTRUCT PARKING AREA SUBGRADE.
- 19. CONSTRUCT CURBS, PAVEMENT STRUCTURE AND SIDEWALKS.
- 20. CONDUCT FINE GRADING.
- 21. PAVING OF PARKING AREAS AND DRIVEWAYS
- 22. FINAL FINE GRADING OF SLOPE AND NON-PAVED AREAS.
- 23. PLACE 4" TOPSOIL ON SLOPES AFTER FINAL GRADING IS COMPLETED. FERTILIZE SEED AND MULCH. INSTALLED APRIL 15 - JUNE 1 OR AUGUST 15 - OCTOBER 1. USE EROSION CONTROL BLANKETS AS ORDERED FOR SLOPES GREATER THAN 3:1 AND AS SHOWN ON LANDSCAPE PLANS OR EROSION CON TEMPORARY STABILIZATION BEYOND SEEDING DATES USE ANNUAL RYE AT 4.0 LBS/1,000 S.F. FERTIL 1.0 LBS. OF NITROGEN PER 1,000 S.F. AND LIME AT 100 LBS/1,000 S.F. (MAX.).
- 24. LANDSCAPE ISLANDS, INTERIOR NON-PAVED AREAS, AND PERIMETER AREAS.
- 25. INSTALL SIGNING AND PAVEMENT MARKINGS
- 26. CLEAN STORM DRAINAGE PIPE STRUCTURES, DETENTION SYSTEMS AND WATER QUALITY DEVICES OF
- 27. UPON DIRECTION OF THE MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION AGENT, SEDIMENT A MEASURES SHALL BE REMOVED FOLLOWING STABILIZATION OF THE SITE.

OPERATION REQUIREMENTS

- CLEARING AND GRUBBING OPERATIONS 1. ALL SEDIMENT AND EROSION CONTROL MEASURES, INCLUDING THE CONSTRUCTION OF TEMPORARY SE AND STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS, WILL BE INSTALLED PRIOR TO THE STA GRUBBING AND DEMOLITION OPERATIONS.
- 2. FOLLOWING INSTALLATION OF ALL SEDIMENT AND EROSION CONTROL MEASURES, THE CONTRACTOR SI WITH GRADING, FILLING OR OTHER CONSTRUCTION OPERATIONS UNTIL THE ENGINEER HAS INSPECTED INSTALLATIONS
- 3. THE CONTRACTOR SHALL TAKE EXTREME CARE DURING CLEARING AND GRUBBING OPERATIONS SO AS UNPROTECTED WETLAND AREAS OR SEDIMENT AND EROSION CONTROL DEVICES.
- 4. FOLLOWING THE COMPLETION OF CLEARING AND GRUBBING OPERATIONS, ALL AREAS SHALL BE STABI AND SEEDING OR CRUSHED STONE AS SOON AS PRACTICAL.

ROUGH GRADING OPERATIONS

- 1. DURING THE REMOVAL AND/OR PLACEMENT OF EARTH AS INDICATED ON THE GRADING PLAN, TOPSO AND APPROPRIATELY STOCKPILED FOR REUSE.
- 2. ALL STOCKPILED TOPSOIL SHALL BE SEEDED, MULCHED WITH HAY, AND ENCLOSED BY A SILTATION
- FILLING OPERATIONS
- 1. PRIOR TO FILLING, ALL SEDIMENT AND EROSION CONTROL DEVICES SHALL BE PROPERLY IMPLEMENTED FULLY INSTALLED, AS DIRECTED BY THE ENGINEER AND AS SHOWN ON THIS PLAN.
- 2. ALL FILL MATERIAL ADJACENT TO ANY WETLAND AREAS, IF APPLICABLE TO THIS PROJECT, SHALL BE LESS THAN 5% FINES PASSING THROUGH A #200 SIEVE (BANK RUN), SHALL BE PLACED IN LIFT THIC GREATER THAN THAT SPECIFIED IN PROJECT SPECIFICATIONS AND/OR THE PROJECT GEOTECHNICAL BE COMPACTED TO 95% MAX. DRY DENSITY MODIFIED PROCTOR OR AS SPECIFIED IN THE CONTRACT THE GEOTECHNICAL REPORT.
- 3. AS GENERAL GRADING OPERATIONS PROGRESS, ANY TEMPORARY DIVERSION DITCHES SHALL BE RAISE NECESSARY, TO DIVERT SURFACE RUNOFF TO THE SEDIMENT BASINS OR SEDIMENT TRAPS. PLACEMENT OF DRAINAGE STRUCTURES, UTILITIES, AND BUILDING CONSTRUCTION OPERATIONS.
- 1. SILT FENCES SHALL BE INSTALLED AT THE DOWNHILL SIDES OF BUILDING EXCAVATIONS, MUD PUMP UTILITY TRENCH MATERIAL STOCKPILES. HAY BALES/STRAW BALES MAY BE USED IF SHOWN ON THE EROSION CONTROL PLANS OR IF DIRECTED BY THE CIVIL ENGINEER.

FINAL GRADING AND PAVING OPERATIONS

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		JUTE MESH, RIP RAP, ETC.) ON-SITE FOR MAINTENANCE AND EMERGENCY REPAIRS.
J STORM DRAINAGE JRSE FROM SEDIMENT CE RUNOFF EROSION	<ol> <li>ALL INLET AND OUTLET PROTECTION SHALL BE PLACED AND MAINTAINED AS SHOWN ON SEDIMENT AND EROSION CONTROL PLANS AND DETAILS, AND AS DESCRIBED IN SPECIFICATIONS AND AS DESCRIBED HEREIN.</li> <li>NO CUT OR FILL SLOPES SHALL EXCEED 2:1 EXCEPT WHERE STABILIZED BY ROCK FACED EMBANKMENTS OR EROSION CONTROL BLANKETS OR JUTE MESH AND VECETATION. ALL SLOPES SHALL BE SEEDED AND ANY ROAD OR DRIVEWAY.</li> </ol>	9. PROTECT EXISTING TREES THAT ARE TO BE SAVED BY FENCING AT THE DRIP LINE OR AS SHOWN WITH SNOW FENCE, ORANGE SAFETY FENCE, OR EQUIVALENT FENCING. ANY LIMB TRIMMING SHOULD BE DONE BEFORE CONSTRUCTION BEGINS IN THAT AREA; FENCING SHALL BE MAINTAINED AND REPAIRED DURING CONSTRUCTION.
FALL 2021. ) BY THE CONTRACTOR	3. PAVEMENT SUB-BASE AND BASE COURSES SHALL BE INSTALLED OVER AREAS TO BE PAVED AS SOON AS FINAL	10. INSTALL PERIMETER SEDIMENT AND EROSION CONTROLS PRIOR TO CLEARING OR CONSTRUCTION. ALL CONSTRUCTION SHALL BE CONTAINED WITHIN THE LIMIT OF DISTURBANCE, WHICH SHALL BE MARKED WITH SILT FENCE, SAFETY FENCE, HAY BALES, RIBBONS, OR OTHER MEANS PRIOR TO CLEARING. CONSTRUCTION ACTIVITY SHALL REMAIN ON THE UPHILL SIDE OF THE SILT FENCE UNLESS WORK IS SPECIFICALLY CALLED FOR ON THE DOWNHILL SIDE OF THE FENCE
L BE REQUIRED TO	SUB-GRADES ARE ESTABLISHED AND UNDERGROUND UTILITIES AND STORM DRAINAGE SYSTEMS HAVE BEEN INSTALLED. 4. AFTER CONSTRUCTION OF PAVEMENT, TOPSOIL, FINAL SEED, MULCH AND LANDSCAPING, REMOVE ALL TEMPORARY SEDIMENT AND EROSION CONTROL DEVICES ONLY AFTER ALL AREAS HAVE BEEN PAVED AND/OR GRASS HAS BEEN WELL ESTABLISHED	11. ANY STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS SHALL BE INSTALLED AT START OF CONSTRUCTION AND MAINTAINED THROUGHOUT THE DURATION OF CONSTRUCTION. THE LOCATION OF THE TRACKING PADS MAY CHANGE AS VARIOUS PHASES OF CONSTRUCTION ARE COMPLETED.
AND WETLANDS NANCE OR ADDITIONAL	AND THE SITE IS STABLE AND HAS BEEN INSPECTED AND APPROVED BY THE MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION. INSTALLATION OF SEDIMENTATION AND EROSION CONTROL MEASURES	12. TOPSOIL SHALL BE STRIPPED AND STOCKPILED FOR USE IN FINAL LANDSCAPING. ALL EARTH STOCKPILES SHALL HAVE HAY BALES OR SILT FENCE AROUND THE LIMIT OF PILE. PILES SHALL BE TEMPORARILY SEEDED IF PILE IS TO REMAIN IN PLACE FOR MORE THAN ONE (1) MONTH.
	I. SILTATION FENCE A. DIG A SIX INCH TRENCH ON THE UPHILL SIDE OF THE DESIGNATED FENCE LINE LOCATION.	13. ANY SEDIMENT BASINS AND SEDIMENT TRAPS SHALL PROVIDE 134 CUBIC YARDS OF SEDIMENT STORAGE PER ACRE CONTRIBUTING TO THE BASIN. PROVIDE BASIN VOLUMES FOR ALL DISTURBANCE ON SITE.
URS PRIOR TO	B. POSITION THE POST AT THE BACK OF THE TRENCH (DOWNHILL SIDE), AND HAMMER THE POST AT LEAST 1.5 FEET INTO THE GROUND.	14. COMPLY WITH REQUIREMENTS OF CGS SECTION 22A 430B, FOR STORMWATER DISCHARGE FROM CONSTRUCTION ACTIVITIES AND WITH DEEP RECORD KEEPING AND INSPECTION REQUIREMENTS.
Y OR INLAND WETLANDS PERIMETER SILT FENCE.	C. LAY THE BOTTOM SIX INCHES OF THE FABRIC INTO THE TRENCH TO PREVENT UNDERMINING BY STORM WATER RUN-OFF.	15. ANY STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS SHALL BE INSTALLED PRIOR TO ANY ON SITE EXCAVATION AND SHALL BE MAINTAINED DURING ALL DEMOLITION, EXCAVATION AND CONSTRUCTION ACTIVITIES.
/EXITS AND INSTALL 'S ON OFF SITE ROADS. ETER OF PROPOSED ED ON THESE PLANS. ERED BY THE ENGINEER	D. BACKFILL THE TRENCH AND COMPACT. II. HAY BALES/STRAW BALES A. BALES SHALL BE PLACED IN A SINGLE ROW, LENGTHWISE, ORIENTED PARALLEL TO THE CONTOUR, WITH ENDS OF ADJACENT BALES TIGHTLY ABUTTING ONE ANOTHER.	16. MINIMIZE LAND DISTURBANCES. SEED AND MULCH DISTURBED AREAS WITH TEMPORARY MIX AS SOON AS PRACTICABLE (ONE WEEK MAXIMUM UNSTABILIZED PERIOD) USING PERENNIAL RYEGRASS AT 40 LBS PER ACRE. MULCH ALL CUT AND FILL SLOPES AND SWALES WITH LOOSE HAY AT A RATE OF 2 TONS PER ACRE. IF NECESSARY, REPLACE LOOSE HAY ON SLOPES WITH EROSION CONTROL BLANKETS OR JUTE CLOTH. MODERATELY GRADED AREAS, ISLANDS, AND TEMPORARY
ONTROLS AT	B. BALES SHALL BE ENTRENCHED AND BACKFILLED. A TRENCH SHALL BE EXCAVATED THE WIDTH OF A BALE AND THE LENGTH OF THE PROPOSED BARRIER TO A MINIMUM DEPTH OF FOUR INCHES. AFTER THE BALES ARE STAKED, THE EXCAVATED SOIL SHALL BE BACKFILLED AGAINST THE BARRIER.	17. MAINTAIN EXISTING PAVED AREAS FOR CONSTRUCTION STAGING FOR AS LONG AS POSSIBLE.
	C. EACH BALE SHALL BE SECURELY ANCHORED BY AT LEAST TWO (2) STAKES.	18. SILT FENCE AND OTHER SEDIMENT AND EROSION CONTROL MEASURES SHALL BE INSTALLED IN ACCORDANCE WITH CONTRACT DRAWINGS AND MANUFACTURER'S RECOMMENDATIONS PRIOR TO WORK IN ANY UPLAND AREAS.
APS. COMMENCE	D. THE GAPS BETWEEN BALES SHALL BE WEDGED WITH STRAW TO PREVENT WATER LEAKAGE.	19. EXCAVATED MATERIAL FROM TEMPORARY SILT TRAPS MUST BE STOCKPILED ON UPHILL SIDE OF SILT FENCE.
SSES AND CONTINUE SITE GRADING. S.	ELEVATION THAN THE TOP OF THE LOWEST MIDDLE BALE, TO ENSURE THAT RUN-OFF WILL FLOW EITHER THROUGH OR OVER THE BARRIER, BUT NOT AROUND IT. OPERATION AND MAINTENANCE OF SEDIMENT AND EROSION	GROUND. SILT FENCE SHALL BE TENCATE ENVIROFENCE, PROPEX GEOTEX OR EQUIVALENT APPROVED BY THE CIVIL ENGINEER. FILTER FABRIC USED SHALL BE TENCATE 140N OR 170N, OR APPROVED EQUIVALENT. SEE SPECIFICATIONS FOR FURTHER INFORMATION.
	CONTROL MEASURES I. SILTATION FENCE A. ALL SILTATION FENCES SHALL BE INSPECTED AS A MINIMUM WEEKLY OR AFTER EACH RAINFALL. ALL DETERIORATED FARBLE AND DAMAGED ROSTS SHALL BE REPLACED AND PROPERLY REPOSITIONED IN ACCORDANCE WITH THIS PLAN	21. WHERE INDICATED ON SEDIMENT AND EROSION CONTROL PLANS USE NEW HAY/STRAW BALES AND REPLACE THEM WHENEVER THEIR CONDITION DETERIORATES BEYOND REASONABLE USABILITY. STAKE BALES SECURELY INTO GROUND AND BUTT TIGHTLY TOGETHER TO PREVENT UNDERCUTTING AND BYPASSING.
E EVIDENCE THAT EACH	B. SEDIMENT DEPOSITS SHALL BE REMOVED FROM BEHIND THE FENCE WHEN THEY REACH A MAXIMUM HEIGHT OF ONE FOOT. II. HAY BALES/STRAW BALES A ALL HAY BALE (STRAW BALE PINGS SHALL BE INSPECTED FOLLOWING FACH BAINFALL BERAIP OF PERIACEMENT SHALL BE	22. INSTALL ANY TEMPORARY DIVERSION DITCHES, PLUNGE POOLS, SEDIMENT BASINS, SEDIMENT TRAPS, CONCRETE WASH PITS AND DEWATERING PITS AS SHOWN AND AS NECESSARY DURING VARIOUS PHASES OF CONSTRUCTION TO CONTROL RUNOFF UNTIL UPHILL AREAS ARE DETERMINED TO BE STABILIZED BY THE AUTHORITY HAVING JURISDICTION. LOCATION OF TEMPORARY SEDIMENT BASINS WILL REQUIRE REVIEW AND APPROVAL BY THE CIVIL ENGINEER AND AUTHORITY HAVING JURISDICTION
ALL ALSO NOTIFY THE RROW AREAS WHEN	PROMPTLY MADE AS NEEDED. B. DEPOSITS SHALL BE REMOVED AND CLEANED-OUT IF ONE HALF OF THE ORIGINAL HEIGHT OF THE BALES BECOMES FILLED WITH SEDIMENT.	23. DIRECT ALL DEWATERING PUMP DISCHARGE TO A SEDIMENT CONTROL DEVICE SUCH AS TEMPORARY PITS, SEDIMENT TRAP, SEDIMENT BASINS OR GRASS FILTERS WITHIN THE APPROVED LIMIT OF DISTURBANCE. DISCHARGE TO STORM DRAINAGE SYSTEM OR SURFACE WATERS FROM SEDIMENT CONTROLS SHALL BE CLEAR.
	III. SEDIMENT BASINS/SEDIMENT TRAPS A. CONTRACTOR TO KEEP WEEKLY CHECKLIST LOGS FOR INSPECTIONS OF ALL SEDIMENT AND EROSION CONTROL DEVICES AND HAVE THEM READILY AVAILABLE ON-SITE AT ALL TIMES FOR INSPECTION BY DEEP, LOCAL AUTHORITIES OR ENGINEER.	24. BLOCK THE OPEN UPSTREAM ENDS OF DETENTION BASIN/SEDIMENTATION BASIN OUTLET CONTROL ORIFICE UNTIL SITE IS STABILIZED. BLOCK END OF STORM SEWERS IN EXPOSED TRENCHES WITH BOARDS AND SANDBAGS AT THE END OF EACH WORKING DAY WHEN RAIN IS EXPECTED.
ALES AND OTHER OVAL SHALL BE ON A	B. ALL SEDIMENT BASINS AND/OR SEDIMENT TRAPS SHALL BE INSPECTED FOLLOWING EACH RAINFALL. REPAIR OF SLOPES SHALL BE PROMPTLY MADE AS NEEDED.	25. SWEEP AFFECTED PORTIONS OF OFF SITE ROADS ONE OR MORE TIMES A DAY (OR LESS FREQUENTLY IF TRACKING IS NOT A PROBLEM) DURING CONSTRUCTION. OTHER DUST CONTROL MEASURES TO BE USED AS NECESSARY INCLUDE WATERING DOWN DISTURBED AREAS, USING CALCIUM CHLORIDE, AND COVERING LOADS ON DUMP TRUCKS.
NT AND EROSION DR GREATER. SEDIMENT	C. SEDIMENT DEPOSITS SHALL BE REMOVED FROM SEDIMENT BASINS AND/OR SEDIMENT TRAPS WHEN THEY REACH A MAXIMUM HEIGHT OF ONE FOOT UNLESS OTHERWISE INDICATED ON THE EROSION CONTROL PLANS AND DETAILS TO BE AT A SPECIFIC ELEVATION PER CLEAN OUT MARKERS.	26. PERIODICALLY CHECK ACCUMULATED SEDIMENT LEVELS IN ANY SEDIMENT BASINS AND SEDIMENT TRAPS DURING CONSTRUCTION AND CLEAN ACCUMULATED SILT WHEN NECESSARY OR WHEN ONE FOOT OF SEDIMENT HAS ACCUMULATED OR PER SPECIFIC CLEANOUT MARKER ELEVATION. CLEAN ACCUMULATED SEDIMENT FROM CATCH BASIN SUMPS AS
	D. SEDIMENT SHALL BE DISPOSED OF ON-SITE OR AS DIRECTED BY THE ENGINEER AND LOCAL GOVERNING OFFICIALS. SEE SEDIMENT AND EROSION CONTROL NOTES HEREIN REGARDING DISPOSAL REQUIREMENTS FOR OFF SITE SPOIL DISPOSAL.	NECESSARY AND AS DIRECTED BY THE CIVIL ENGINEER OR OWNER'S CONSTRUCTION REPRESENTATIVE. REMOVE ACCUMULATED SEDIMENT FROM BEHIND HAY/STRAW BALES AND SILT FENCE WHEN LEVEL REACHES HALF THE HEIGHT OF THE BALE OR ONE FOOT AT SILT FENCE. DISPOSE OF SEDIMENT LEGALLY EITHER ON OR OFF SITE.
	<ol> <li>SEDIMENT AND EROSION CONTROL PLAN</li> <li>HAY BALE/STRAW BALE FILTERS WILL BE INSTALLED AT ALL CULVERT OUTLETS IF CULVERT OUTLETS ARE APPLICABLE TO THIS PROJECT AND SILTATION FENCE INSTALLED ALONG THE TOE OF ALL CRITICAL CUT AND FILL SLOPES.</li> <li>CULVERT DISCHARGE AREAS WILL BE PROTECTED WITH RIP RAP CHANNELS. ENERGY DISSIPATORS WILL BE INSTALLED AS</li> </ol>	27. IMMEDIATELY UPON DISCOVERING UNFORESEEN CIRCUMSTANCES POSING THE POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION, THE OPERATOR SHALL IMPLEMENT APPROPRIATE BEST MANAGEMENT PRACTICES TO ELIMINATE THE POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION.
	SHOWN ON THESE PLANS AND AS NECESSARY. 3. CATCH BASINS WILL BE PROTECTED WITH HAY BALE/STRAW BALE FILTERS, SILT SACKS, SILTATION FENCE, OR OTHER INLET PROTECTION DEVICES PER DETAILS. THROUGHOUT THE CONSTRUCTION PERIOD AND UNTIL ALL DISTURBED AREAS ARE	28. ALL PUMPING OF SEDIMENT LADEN WATER SHALL BE THROUGH A SEDIMENT CONTROL BMP, SUCH AS A PUMPED WATER FILTER BAG OR EQUIVALENT SEDIMENT REMOVAL FACILITY, OVER UNDISTURBED VEGETATED AREAS.
SEED MIXTURE TO BE	THOROUGHLY STABILIZED.	29. ALL EXCAVATED MATERIAL SHALL BE PLACED ON THE HIGH SIDE OF UTILITY AND STORM PIPE TRENCHES SU AS TO ALLOW THE TRENCH TO INTERCEPT ALL SILT LADEN RUNOFF.
S REQUIRED OR NTROL PLANS. FOR LIZE WITH 10-10-10 AT	SPECIFICATIONS OF THE 2002 CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL, LATEST EDITION.	30. CONTRACTOR SHALL ONET EXCAVATE AS MOCH UTILITY AND STORM FIFE TRENCH WORK AS CAN BE COMPLETED, BACKFILLED AND STABILIZED IN ONE DAY SO AS TO LIMIT THE AMOUNT OF OPEN, DISTURBED TRENCHING.
	<ul> <li>SEDIMENT AND EROSION CONTROL MEASURES WILL BE INSTALLED PRIOR TO DEMOLITION AND/OR CONSTRUCTION WHENEVER POSSIBLE.</li> <li>ALL CONTROL MEASURES WILL BE MAINTAINED IN EFFECTIVE CONDITION THROUGHOUT THE DEMOLITION AND CONSTRUCTION PERIOD UNTIL THE SITE IS DETERMINED TO BE STABILIZED BY THE AUTHORITY HAVING JURISDICTION.</li> </ul>	STABILIZE POTENTIALLY WIND-BLOWN MATERIAL. HAUL ROADS BOTH INTO AND AROUND THE SITE ARE TO BE SPRAYED AS NEEDED TO SUPPRESS DUST. TRUCKS HAULING IMPORT FILL MATERIAL ARE TO BE TARPED TO AID IN THE CONTROL OF AIRBORNE DUST. DURING HIGH WIND EVENTS (20 TO 30 MPH SUSTAINED) CONSTRUCTION ACTIVITY SHALL BE LIMITED OR CEASED IF DUST CANNOT BE CONTROLLED BY WETTING
DEBRIS AND SEDIMENT.	7. ADDITIONAL CONTROL MEASURES WILL BE INSTALLED DURING THE CONSTRUCTION PERIOD, IF NECESSARY OR REQUIRED OR AS DIRECTED BY THE CIVIL ENGINEER OR BY THE AUTHORITY HAVING JURISDICTION.	32. AN AREA SHALL BE CONSIDERED TO HAVE ACHIEVED FINAL STABILIZATION WHEN IT HAS A MINIMUM OF 70% UNIFORM PERENNIAL VEGETATIVE COVER OR OTHER PERMANENT NON-VEGETATIVE COVER WITH A DENSITY SUFFICIENT TO RESIST
ND EROSION CONTROL	8. SEDIMENT REMOVED FROM EROSION CONTROL STRUCTURES WILL BE DISPOSED IN A MANNER WHICH IS CONSISTENT WITH THE INTENT AND REQUIREMENTS OF THE SEDIMENT AND EROSION CONTROL PLANS, NOTES, AND DETAILS.	ACCELERATED SURFACE EROSION AND SUBSURFACE CHARACTERISTICS SUFFICIENT TO RESIST SLIDING OR OTHER MOVEMENTS UNLESS OTHERWISE DETERMINED BY THE AUTHORITY HAVING JURISDICTION.
EDIMENTATION BASINS ART OF CLEARING AND	9. THE CONTRACTOR IS ASSIGNED THE RESPONSIBILITY FOR IMPLEMENTING THIS SEDIMENT AND EROSION CONTROL PLAN. THIS RESPONSIBILITY INCLUDES THE INSTALLATION AND MAINTENANCE OF CONTROL MEASURES, INFORMING ALL PARTIES ENGAGED ON THE CONSTRUCTION SITE OF THE REQUIREMENTS AND OBJECTIVES OF THE PLAN, NOTIFICATION OF THE MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION OFFICE OR AUTHORITY HAVING JURISDICTION OF ANY TRANSFER OF THIS RESPONSIBILITY AND FOR CONVEYING A COPY OF THE SEDIMENT AND EROSION CONTROL PLAN IF THE TITLE TO THE LAND IS TRANSFERRED.	33. MAINTAIN ALL PERMANENT AND TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES IN EFFECTIVE CONDITION THROUGHOUT THE CONSTRUCTION PERIOD. UPON COMPLETION OF WORK SWEEP PARKING LOT AND REMOVE ALL TEMPORARY EROSION AND SEDIMENT CONTROLS WHEN AUTHORIZED BY AUTHORITY HAVING JURISDICTION. FILE NOT (NOTICE OF TERMINATION) WITH AUTHORITY HAVING JURISDICTION RESPONSIBLE FOR REGULATING STORM WATER DISCHARGES FROM CONSTRUCTION ACTIVITIES PER NPDES. STATE SPECIAL CONCERN SPECIES
SHALL NOT PROCEED	SEDIMENT AND EROSION CONTROL NOTES 1. THE SEDIMENT AND EROSION CONTROL PLAN IS ONLY INTENDED TO DESCRIBE THE SEDIMENT AND EROSION CONTROL TREATMENT FOR THIS SITE. SEE SEDIMENT AND EROSION CONTROL DETAILS AND CONSTRUCTION SEQUENCE. REFER TO SITE DAY FOR THIS SITE. SEE SEDIMENT AND EROSION CONTROL DETAILS AND CONSTRUCTION SEQUENCE. REFER TO	1. CONTRACTOR IS RESPONSIBLE FOR HIRING A QUALIFIED HERPETOLOGIST TO WORK WITH CONSTRUCTION CREW TO ENSURE THAT TURTLES WILL NOT BE UNINTENTIONALLY KILLED DURING THE MOVING OF HEAVY EQUIPMENT, ESPECIALLY IN THE MONTH OF JUNE.
S NOT TO DISTURB	2. THE CONTRACTOR IS RESPONSIBLE FOR IMPLEMENTING THIS SEDIMENT AND EROSION CONTROL PLAN. THIS RESPONSIBILITY INCLUDES THE PROPER INSTALLATION AND MAINTENANCE OF SEDIMENT AND EROSION CONTROL MEASURES. INFORMING ALL	2. THE LIMIT OF DISTURBANCE SHALL BE FENCED WITH EXCLUSIONARY FENCING THAT IS SECURED AND IN CONTACT WITH THE GROUND AND AT LEAST 20INCHES HIGH. THE FENCE SHALL BE MAINTAINED BI-WEEKLY AND AFTER MAJOR WEATHER EVENTS. DO NOT USE PLASTIC NETTED OR NETTED SILT FENCE.
BILIZED WITH TOPSOIL	PARTIES ENGAGED WITH CONSTRUCTION ON THE SITE OF THE REQUIREMENTS AND OBJECTIVES OF THIS PLAN, INFORMING THE AUTHORITY HAVING JURISDICTION OR COUNTY OR INLAND WETLANDS AGENCY OF ANY TRANSFER OF THIS RESPONSIBILITY, AND FOR CONVEYING A COPY OF THE SEDIMENT & EROSION CONTROL PLAN IF THE TITLE TO THE LAND IS TRANSFERRED	3. ALL STAGING AND STORAGE AREAS, OUTSIDE OF PREVIOUSLY PAVED LOCATIONS, REGARDLESS OF THE DURATION OF TIME THEY WILL BE UTILIZED, MUST BE REVIEWED TO REMOVE INDIVIDUALS AND EXCLUDE THEM FROM RE-ENTRY.
OIL SHALL BE STRIPPED	IRANSFERRED. 3. AN EROSION CONTROL BOND MAY BE REQUIRED TO BE POSTED WITH THE MUNICIPALITY TO ENSURE IMPLEMENTATION OF THE SEDIMENT AND EROSION CONTROL MEASURES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE POSTING OF THIS BOND AND FOR INQUIRIES TO THE MUNICIPALITY FOR INFORMATION ON THE METHOD, TYPE AND AMOUNT OF THE BOND POSTING UNLESS OTHERWISE DIRECTED BY THE OWNER.	<ol> <li>ALL CONSTRUCTION PERSONNEL WORKING WITHIN THE TURTLE HABITAT MUST BE APPRISED OF THE SPECIES DESCRIPTION AND THE POSSIBLE PRESENCE OF A LISTED SPECIES, AND INSTRUCTED TO RELOCATE TURTLES FOUND INSIDE WORK AREAS OR NOTIFY THE APPROPRIATE AUTHORITIES TO RELOCATE INDIVIDUALS.</li> <li>ANY TURTLES ENCOUNTERED WITHIN THE IMMEDIATE WORK AREA SHALL BE CAREELULY MOVED TO AN ADJACENT AREA</li> </ol>
FENCE.	<ul> <li>4. VISUAL SITE INSPECTIONS SHALL BE CONDUCTED WEEKLY, AND AFTER EACH MEASURABLE PRECIPITATION EVENT OF 0.25 INCHES OR GREATER BY QUALIFIED PERSONNEL, TRAINED AND EXPERIENCED IN SEDIMENT AND EROSION CONTROL, TO</li> </ul>	6 IN AREAS WHERE SILT FENCE IS LISED FOR EXCLUSION IT SHALL BE REMOVED AS SOON AS THE AREA IS STARLE TO
ED, MAINTAINED AND	ASCERTAIN THAT THE SEDIMENT AND EROSION CONTROL (E&S) BMPS ARE OPERATIONAL AND EFFECTIVE IN PREVENTING POLLUTION. A WRITTEN REPORT OF EACH INSPECTION SHALL BE KEPT, AND INCLUDE: A)A SUMMARY OF THE SITE CONDITIONS, E&S BMPS, AND COMPLIANCE: AND	ALLOW FOR REPTILE AND AMPHIBIAN PASSAGE TO RESUME. 7. NO HEAVY MACHINERY OR VEHICLES MAY BE PARKED IN ANY TURTLE HABITAT.
E GOOD QUALITY, WITH	B) THE DATE, TIME, AND THE NAME OF THE PERSON CONDUCTING THE INSPECTION	8. SPECIAL PRECAUTIONS MUST BE TAKEN TO AVOID DEGRADATION OF WETLAND HABITATS INCLUDING ANY WET MEADOWS AND SEASONAL POOLS.
ICKNESSES NOT REPORT. LIFTS SHALL SPECIFICATIONS OR IN	GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL, LATEST EDITION IN ACCORDANCE WITH THE CONTRACT DOCUMENTS, AND AS DIRECTED BY THE MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION. THE CONTRACTOR SHALL KEEP A COPY OF THE GUIDELINES ON-SITE FOR REFERENCE DURING CONSTRUCTION.	9. THE CONTRACTOR AND CONSULTING HERPETOLOGIST MUST SEARCH THE WORK AREA EACH MORNING PRIOR TO ANY WORK BEING DONE.
SED OR LOWERED, AS	6. ADDITIONAL AND/OR ALTERNATIVE SEDIMENT AND EROSION CONTROL MEASURES MAY BE INSTALLED DURING THE CONSTRUCTION PERIOD IF FOUND NECESSARY BY THE CONTRACTOR, OWNER, SITE ENGINEER, MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION, OR GOVERNING AGENCIES. THE CONTRACTOR SHALL CONTACT THE OWNER AND APPROPRIATE GOVERNING AGENCIES FOR APPROVAL IF ALTERNATIVE CONTROLS OTHER THAN THOSE SHOWN ON THE PLANS ARE PROPOSED.	<ol> <li>WHEN FELLING TREES ADJACENT TO BROOKS AND STREAMS PLEASE CUT THEM TO FALL AWAY FROM THE WATERWAY AND DO NOT DRAG TREES ACROSS THE WATERWAY OR REMOVE STUMPS FROM BANKS.</li> <li>AVOID AND LIMIT ANY EQUIPMENT USE WITHIN 50 FEET OF STREAMS AND BROOKS.</li> </ol>
DISCHARGES, AND SEDIMENT AND	7. THE CONTRACTOR SHALL INSPECT ALL SEDIMENT AND EROSION CONTROLS BEFORE AND AFTER EACH STORM (0.25 INCHES OR GREATER RAINFALL), OR AT LEAST WEEKLY. TO VERIFY THAT THE CONTROLS ARE OPERATING PROPERLY AND MAKE	12. ANY CONFIRMED TURTLE SIGNTINGS SHOULD BE REPORTED TO THE NATURAL DIVERSITY DATA BASE AT (nddbrequestdep@ct.gov) USING REPORTING FORMS FOUND ON THE NDDB WEBPAGE

8. THE CONTRACTOR SHALL KEEP A SUPPLY OF SEDIMENT AND EROSION CONTROL MATERIAL (ANY HAY BALES, SILT FENCE,

REPAIRS WHERE NECESSARY.

### FOR PERMITTING PURPOSES ONLY NOT RELEASED FOR CONSTRUCTION

THE LIMIT OF DISTURBANCE, WHICH SHALL BE MARKED WITH SILT FENCE, SAFETY FENCE, HAY BALES, IEANS PRIOR TO CLEARING. CONSTRUCTION ACTIVITY SHALL REMAIN ON THE UPHILL SIDE OF THE

, OR EQUIVALENT FENCING. ANY LIMB TRIMMING SHOULD BE DONE BEFORE CONSTRUCTION BEGINS EDIMENT AND EROSION CONTROLS PRIOR TO CLEARING OR CONSTRUCTION. ALL CONSTRUCTION SHALL

100 Constitution Plaza 10th Floor Hartford, CT 06103 (860) 249-2200



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Project No.

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SEDIMENT AND

CONTROL NOTES

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04/02/2021

LAN	LANDSCAPE ZONING INFORMATION						
LOCATIO	LOCATION: BOLTON, TOLLAND COUNTY, CONNECTICUT						
ZONE:	ZONE: RURAL MIXED USE ZONE (RMUZ)						
USE: F	USE: RETAIL (PERMITTED BY SPECIAL PERMIT)						
ITEM #	ITEM	REQUIREMENTS	PROPOSED	VARIANCE			
1	LANDSCAPE AREA (SEC.11.J)	NO LESS THAN 30% OF AN RMUZ ZONE SHALL BE LANDSCAPED. SIDEWALKS ARE EXCLUDED FROM LANDSCAPE AREA	GREATER THAN 30% LANDSCAPED	NO			
2	LANDSCAPE PARKING (SEC.15.H)	INTERIOR LANDSCAPING SHALL BE PROVIDED AT A RATE OF 20 SF PER PARKING SPACE. LANDSCAPING SHALL BE WITHIN RAISED, CURBED ISLANDS. (20 SF X 33 SPACES = 660 SF)	730 SF PROPOSED	NO			
3	LANDSCAPE PARKING (SEC.15.H)	PARKING AREAS ABUTTING A RESIDENTIAL ZONE SHALL BE SCREENED BY A 10' WIDTH EVERGREEN ROW. PLANTS TO BE 4' HT AND 4' O.C. AT TIME OF PLANTING.	COMPLIES	NO			
4	LANDSCAPE REQUIREMENTS (SEC.16A.3.q.3)	INTERIOR LANDSCAPE AREAS SHALL BE 100 SF MIN AND 8' WIDTH MIN.	COMPLIES	NO			
5	LANDSCAPE REQUIREMENTS (SEC.16A.3.q.3)	INTERIOR AREAS SHALL HAVE 1 TREE PER 20 PARKING SPACES	COMPLIES	NO			
6	LANDSCAPE REQUIREMENTS (SEC.16A.3.q.3)	PARKING PERIMETER LANDSCAPE AREA SHALL BE 5' WIDTH MIN. WITH 1 TREE PER 50 LF	COMPLIES	NO			
7	LANDSCAPE REQUIREMENTS (SEC.16A.3.q.3)	TREES TO BE 3" CAL. AND 10' HT. MIN. AT TIME OF PLANTING	COMPLIES	NO			
8	STREET PLANTINGS (SEC.16A.3.q.4)	LANDSCAPE ADJACENT TO STREET TO BE 30' WDTH WITH 1 TREE PER 40' LOT LINE FRONTAGE (260 LF FRONTAGE ÷ 40 = 6.5 TREES)	5 TREES PROPOSED, 2 TREES TO REMAIN	NO			
9	LANDSCAPE DESIGN (SEC.16A.3.x.3.g.10)	FOR EVERY 5 PARKING SPACES, 1 TREE SHALL BE PROVIDED (33 PARKING SPACES $\div$ 5 = 6.6 TREES)	GREATER THAN 7 TREES PROVIDED	NO			
10	LANDSCAPE DESIGN GUIDELINES (CH.8.1.3)	PLANT MATERIAL TO BE INDIGENOUS TO THE AREA, OR IF NOT NATIVE, THAN HARDY AND NON-INVASIVE	COMPLIES	NO			
11	LANDSCAPE DESIGN GUIDELINES (CH.8.1.20&21)	ALL PLANTINGS SHALL BE GUARANTEED FOR 2 YEARS MINIMUM. A COPY OF THE GUARANTEE CONTRACT SHALL BE SUBMITTED TO THE TOWN.	SEE LANDSCAPE NOTE #4 ON SHEET LL-2	NO			
12	LANDSCAPE DESIGN GUIDELINES (CH.8.1.22)	FLOWERING TREES TO BE 2"-2.5" CAL./DECIDUOUS TREES 3"-3.5" CAL./EVERGREEN TREES TO BE 5'-'7 HT. MIN/DECIDUOUS SHRUBS 24" HT./EVERGREEN SHRUBS 18" HT./PERENNIALS 1 GAL. CONT.	COMPLIES	NO			

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LANDS	CAPE P	LANT SCHEDULE					
TREES							
KEY	QTY	BOTANICAL NAME	COMMON NAME	ROOT	SIZE AT INSTALL	SIZE AT MATURITY	COMMENTS
AR	3	Acer rubrum 'Franksred'	RED SUNSET RED MAPLE	B&B	3" CAL. MIN.	45' × 35'	7' BRANCH HT. MIN.
AS	3	Acer saccharum	SUGAR MAPLE	B&B	3" CAL. MIN.	45' × 40'	7' BRANCH HT. MIN.
BN	3	Betula nigra 'Cully'	HERITAGE RIVER BIRCH	B&B	10' HT. MIN.	40' × 30'	MULTI-STEM
со	4	Celtis occidentalis 'Prairie Pride'	PRAIRIE PRIDE HACKBERRY	B&B	3" CAL. MIN.	45' x 35'	7' BRANCH HT. MIN.
PG	3	Picea glauca	WHITE SPRUCE	B&B	6' HT. MIN.	50' x 15'	FULL BRANCHING TO GROUND
PS	8	Pinus strobus	EASTERN WHITE PINE	B&B	6' HT. MIN.	60' x 30'	FULL BRANCHING TO GROUND
PA	4	Platanus x acerifolia 'Morton's Circle'	EXCLAMATION! PLANETREE	B&B	3" CAL. MIN.	55' x 35'	7' BRANCH HT. MIN.
QR	3	Quercus rubra	RED OAK	B&B	3" CAL. MIN.	50' × 45'	7' BRANCH HT. MIN.
QP	3	Quercus palustris	PIN OAK	B&B	3" CAL. MIN.	55' × 40'	7' BRANCH HT. MIN.
TG	20	Thuja 'Green Giant'	GREEN GIANT ARBORVITAE	B&B	6' HT. MIN.	50' x 15'	FULL BRANCHING TO GROUND
SHRUB	S						
cs	7	Cornus sericea 'Arctic Fire'	ARCTIC FIRE REDTWIG DOGWOOD	CONT.	24" HT. MIN.	3.5' x 3.5'	PLANT 4' O.C.
IG	17	llex glabra	INKBERRY	CONT.	4' HT. MIN.	7' x 6'	PLANT 4' O.C.
IGC	27	llex glabra 'Compacta'	COMPACT INKBERRY	CONT.	24" HT. MIN.	4' × 5'	PLANT 4' O.C.
MP	7	Myrica pensylvanica	BAYBERRY	CONT.	30" HT. MIN.	8' x 8'	PLANT 5' O.C.
RC	8	Rhododendron 'Cunningham's White'	CUNNINGHAM'S WHITE RHODODENDRON	CONT.	24" HT. MIN.	3' × 4'	PLANT 4' O.C.
RH	6	Rhododendron 'Lavender Princess'	LAVENDER PRINCESS RHODODENDRON	CONT.	24" HT. MIN.	4' x 5'	PLANT 4' O.C.
ORNAM	IENTAL	GRASSES					•
PV	24	Panicum virgatum 'Shenandoah'	SHENANDOAH SWITCHGRASS	CONT.	24" HT. MIN.	4' × 2'	PLANT 30" O.C.
SH	27	Sporobolus heterolepis	PRARIE DROPSEED	CONT.	12" HT. MIN.	2.5' x 2.5'	PLANT 30" O.C.
PEREN	NIALS A	ND GROUNDCOVERS		-			
АМ	15	Aronia melanocarpa 'UCONNAM165'	LOW SCAPE MOUND CHOKEBERRY	CONT.	12" HT. MIN.	2' × 3'	PLANT 30" O.C.
cv	10	Coreopsis verticillata 'Grandiflora'	GRANDIFLORA COREOPSIS	CONT.	8" HT. MIN./ 1 GAL. CONT.	2.5' x 2.5'	PLANT 30" O.C.
RF	16	Rudbeckia fulgida 'Goldstrum'	BLACK-EYED SUSAN	CONT.	8" HT. MIN./ 1 GAL. CONT.	2.5' x 2.5'	PLANT 30" O.C.



NOTES: 1) ALL SUBSTITUTIONS MUST RECEIVE APPROVAL FROM THE LANDSCAPE ARCHITECT PRIOR TO DELIVERY TO SITE.

PROVIDE AND INSTALL ALL PLANTS SHOWN ON THE PLANTING PLAN DRAWINGS; THE QUANTITIES IN THE PLANT LIST ARE PROVIDED FOR THE CONTRACTOR'S CONVENIENCE ONLY. IF DISCREPANCIES OCCUR, THE LARGER QUANTITY SHALL APPLY.

CHAIRMAN

21 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES.

3) IF THERE IS A DISCREPANCY BETWEEN BOTANICAL AND COMMON NAME, BOTANICAL NAME PREVAILS.

# SEE SHEET LL-2 FOR LANDSCAPE NOTES AND DETAILS

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED DATE OF EXPIRATION

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON

# LEGEND

# <u>PATTERN</u>



DESCRIPTION

SOD (REFER TO SEED MIXES ON SHEET LL-2)



2021 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES

THE LANDSCAPE PLAN AND DETAIL SHEET ARE FOR LANDSCAPING INFORMATION ONLY. REFER TO THE OTHER PLANS FOR ALL OTHER

COORDINATE PLANT MATERIAL LOCATIONS WITH SITE UTILITIES. UTILITY LOCATIONS SHOWN ON THE DRAWINGS ARE APPROXIMATE. EXERCISE CARE WHEN DIGGING IN AREAS OF POTENTIAL CONFLICT WITH UNDERGROUND OR OVERHEAD UTILITIES. THE CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE DUE TO CONTRACTOR'S NEGLIGENCE AND SHALL REPLACE OR REPAIR ANY DAMAGE AT CONTRACTOR'S EXPENSE. PRIOR TO DIGGING AND INSTALLATION OF PLANT MATERIAL, THE CONTRACTOR SHALL CONTACT "CALL BEFORE YOU DIG" 72 HOURS BEFORE COMMENCEMENT OF WORK AT "(800) 922-4455" AND VERIFY ALL UTILITY SYSTEM LOCATIONS.

THE LOCATIONS FOR PLANT MATERIAL ARE APPROXIMATE AND ARE SUBJECT TO FIELD ADJUSTMENT DUE TO UTILITY LOCATIONS AND SITE CONDITIONS. THE CONTRACTOR SHALL ACCURATELY STAKE OUT THE LOCATIONS FOR ALL PLANTS FOR THE REVIEW, ADJUSTMENT, AND APPROVAL BY OWNER OR LANDSCAPE ARCHITECT PRIOR TO PLANTING.

4. THE CONTRACTOR SHALL GUARANTEE THAT ALL PLANTS SHALL BE HEALTHY AND FREE OF DISEASE FOR A PERIOD OF TWO YEARS AFTER SUBSTANTIAL COMPLETION AND ACCEPTANCE BY OWNER OR LANDSCAPE ARCHITECT. CONTRACTOR SHALL REPLACE ANY DEAD OR UNHEALTHY PLANTS AT CONTRACTOR'S EXPENSE. PLANT MATERIAL REPLACEMENTS SHALL BE GUARANTEED FOR TWO FULL YEARS FROM DATE OF REPLACEMENT. REPLACEMENT PLANTS SHALL BE THE SAME AS SPECIFIED FOR THE ORIGINAL PLANTING. REPLACEMENTS SHALL BE MADE AS MANY TIMES AS NECESSARY TO ENSURE HEALTHY PLANTS. FINAL ACCEPTANCE SHALL BE MADE IF ALL PLANTS MEET THE GUARANTEE REQUIREMENTS INCLUDING MAINTENANCE. MAINTENANCE RESPONSIBILITIES INCLUDE CULTIVATING, SPRAYING, WEEDING, WATERING, TIGHTENING GUYS, PRUNING, FERTILIZING, MULCHING, AND ANY OTHER OPERATIONS NECESSARY TO MAINTAIN PLANT VIABILITY. MAINTENANCE SHALL BEGIN IMMEDIATELY AFTER PLANTING AND CONTINUE UNTIL THE END OF THE GUARANTEE PERIOD. DURING THE LANDSCAPE MAINTENANCE PERIOD (GUARANTEE) THE LANDSCAPE CONTRACTOR SHALL NOTIFY THE OWNER IN WRITING OF ANY SITE CONSTRAINTS (PHYSICAL, ENVIRONMENT, ETC.) OR MAINTENANCE DEFICIENCIES THAT MAY AFFECT LANDSCAPE VEGETATION ESTABLISHMENT.

THE CONTRACTOR SHALL SUPPLY ALL LABOR, PLANTS, AND MATERIALS IN QUANTITIES SUFFICIENT TO COMPLETE THE WORK SHOWN ON THE DRAWINGS AND LISTED IN THE PLANT SCHEDULE. IN THE EVENT OF A DISCREPANCY BETWEEN QUANTITIES SHOWN IN THE PLANT SCHEDULE AND THOSE REQUIRED BY THE DRAWINGS, THE LARGER SHALL APPLY. ALL PLANTS SHALL BE ACCLIMATED BY THE SUPPLY NURSERY TO THE LOCAL HARDINESS ZONE AND BE CERTIFIED THAT THE PLANTING MATERIAL HAS BEEN GROWN FOR A MINIMUM OF TWO YEARS AT THE SOURCE AND OBTAINED WITHIN 200 MILES OF PROJECT SITE UNLESS OTHERWISE APPROVED BY OWNER OR LANDSCAPE

6. PLANTS SHALL HAVE TAGS THAT IDENTIFY PLANT GENUS, SPECIES, CULTIVAR (IF APPLICABLE), PLANT COMMON NAME, NAME OF SOURCE NURSERY, AND SIZE OF PLANT FOR REVIEW OF OWNER OR LANDSCAPE ARCHITECT.

NO PLANT SHALL BE PLACED IN THE GROUND BEFORE ROUGH GRADING HAS BEEN COMPLETED AND APPROVED BY THE OWNER OR LANDSCAPE ARCHITECT. STAKING THE LOCATION OF ALL TREES AND SHRUBS SHALL BE COMPLETED PRIOR TO PLANTING FOR APPROVAL BY

8. FINAL GRADES SHALL BLEND SMOOTHLY WITH EXISTING GRADES, AND TOP AND BOTTOM OF SLOPES SHALL BE ROUNDED.

9. ALL TREE AND SHRUB MASSINGS SHALL BE MULCHED TO A DEPTH OF 3". ANNUAL AND PERENNIAL BEDS SHALL BE MULCHED TO A DEPTH OF 2". MULCH SHALL BE UNCOLORED TRIPLE-SHREDDED HARDWOOD BARK MULCH, AGED AT LEAST 6 MONTHS.

10. IF TREE STAKING IS PROPOSED, TREE STAKING MUST BE COMPLETED THE SAME DAY AS THE TREE IS INSTALLED. ALL TREES SHALL BE

11. LANDSCAPE PLANTING AREAS MUST BE FREE DRAINING. PAVEMENT, COMPACTED SUBGRADE, DEAD OR DYING PLANT MATERIAL, BLASTED ROCK, STONES GREATER THAN 1" IN DIAMETER, AND ANY OTHER MATERIAL HARMFUL TO PLANT GROWTH AND DEVELOPMENT SHALL BE REMOVED FROM AREAS TO BE LANDSCAPED AS REQUIRED BY PLANTING DETAILS OR SPECIFICATIONS.

DEPTH: PLANTING SOIL SHALL BE INSTALLED AT A MINIMUM DEPTH OF 4" OR AS NOTED IN THE LANDSCAPE DETAILS. PLANTING SOIL SHALL BE UTILIZED IN ALL PLANTING AREAS INCLUDING SEEDED AREAS.

TESTING: CONTRACTOR SHALL SUBMIT (2) SOIL SAMPLES PER SOIL STOCKPILE TO A CERTIFIED TESTING LABORATORY TO DETERMINE ACIDITY. ORGANIC CONTENT, MECHANICAL ANALYSIS, AVAILABLE NUTRIENTS (N,P,K,Ca,Mg,S,Fe,Mn,Zn,Cu,B,AI,Pb) AND NECESSARY AMENDMENTS TO SOIL. THE CONTRACTOR SHALL SUBMIT THE TEST RESULTS TO THE OWNER OR LANDSCAPE ARCHITECT FOR REVIEW AND APPROVAL. TEST RESULTS SHALL RECOMMEND AMENDMENTS THAT WILL ALTER THE SOIL CHARACTERISTICS SUCH THAT THE CHARACTERISTICS DESCRIBED BELOW ARE ACHIEVED AND THE SPECIFIED PLANTS (CONTRACTOR TO PROVIDE LIST TO TESTING LABORATORY) WILL ACHIEVE PROPER GROWTH THAT IS NEITHER DEFICIENT NOR EXCESSIVE. THE CONTRACTOR SHALL INCORPORATE THESE AMENDMENTS

CHARACTERISTICS: PLANTING SOIL MAY CONSIST OF EXISTING ON-SITE SOILS, AMENDED ON-SITE SOILS, OR IMPORTED SOILS MEETING THE

A. NOT TO CONTAIN MATERIALS HARMFUL TO PLANT LIFE, TO BE CLEAN, FERTILE, FRIABLE, AND WELL DRAINING. ALL PLANTING SOIL SHALL BE FREE OF ANY SUBSOIL EARTH CLODS, SODS, STONES OVER 1" IN ANY DIMENSION, STICKS, ROOTS, WEEDS, LITTER AND OTHER DELETERIOUS MATERIAL. PLANTING SOIL SHALL BE UNIFORM IN QUALITY AND TEXTURE. B. PLANTING SOIL SHALL HAVE THE FOLLOWING OPTIMUM RANGES UNLESS OTHERWISE APPROVED BY THE OWNER OR LANDSCAPE ARCHITECT.

3% - 6% FOR LAWN OR GRASS AREAS. ORGANIC CONTENT 4% - 8% FOR TREE AND SHRUB PLANTERS.

8%-16% FOR RETENTION OR DETENTION BASINS. (BY LOSS OF IGNITION AT 375 C METHOD OF TESTING)

6.0 - 7.3

C. NUTRIENT LEVELS SHALL BE ACHIEVED BY THE CONTRACTOR'S ADDITION OF AMENDMENTS TO THE PLANTING SOIL TO MEET THE OPTIMUM NUTRIENT LEVELS SPECIFIED IN THE TESTING LABORATORY REPORT FOR EACH OF PLANTS TO BE INSTALLED. D. SOIL SHALL BE COMPACTED TO A SURFACE PENETRATION RESISTANCE OF 75-125 LBS/SQ.IN. SOIL MAY BE TREATED FOR WEEDS WITH PRE-EMERGENT OR POST-EMERGENT HERBICIDE, AS NEEDED AND AS APPROPRIATE FOR THE APPLICATION SEASON OR LOCATION, OR ELIMINATE GROWTH OF UNWANTED PLANT MATERIAL. APPLY HERBICIDES IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS. HERBICIDE APPLICATOR MUST BE LICENSED IN THE STATE OF CONNECTICUT, AND PERFORM APPLICATIONS IN ACCORDANCE WITH LOCAL REQUIREMENTS, PERMITTING STIPULATIONS, AND ANY OTHER RESTRICTIONS INCLUDING AND IN EXCESS OF STATE AND FEDERAL REGULATIONS. F. PROPOSED TOPSOIL SHALL MEET THE USDA SOILS TEXTURAL PERCENTAGES OF SAND, SILT, AND CLAY FOR FOLLOWING CLASSIFICATIONS:

– SANDY LOAM WHERE SAND DOES NOT EXCEED 70% AND CLAY IS NOT LESS THAN 5% - SANDY CLAY LOAM WHERE SAND DOES NOT EXCEED 70% AND CLAY IS LESS THAN 28%.

G. BIORETENTION SOILS: SOIL TO BE INSTALLED IN RETENTION BASINS, PONDS, OR OTHER STORMWATER MANAGEMENT ENVIRONS SHALL MEET THE ABOVE DESCRIBED CHARACTERISTICS AND AS FOLLOWS: - SOIL SHALL NOT CONTAIN MORE THAN 20% CLAY AND LESS THAN 40% SILT.

- SOIL SHALL HAVE AN INFILTRATION RATE BETWEEN 1/2" AND 3" PER HOUR.

H. MODIFICATION TO THE PLANTING SOIL CHARACTERISTICS DESCRIBED ABOVE MAY BE SUBMITTED FOR APPROVAL BY THE LANDSCAPE ARCHITECT. CONTRACTOR MUST DEMONSTRATE PROPOSED CHARACTERISTICS ARE EQUAL TO OR SUPERIOR TO THE SPECIFIED CHARACTERISTICS WITH RESPECT TO SUPPORTING PLANT GROWTH, AND STORMWATER MANAGEMENT.

APPLY FERTILIZER AND OTHER AMENDMENTS AS RECOMMENDED FOR EACH PLANTING AREA BY SOIL ANALYSIS. APPLY AMENDMENTS IN A MANNER CONSISTENT WITH MANUFACTURER'S RECOMMENDATIONS. ANY ORGANIC AMENDMENTS SHALL HAVE A DH BETWEEN 4.5 AND 5.5

13. PLANT REQUIREMENTS: ALL PLANTS SHALL CONFORM IN SIZE AND GRADE TO THE AMERICAN STANDARD FOR NURSERY STOCK, ANSI Z60.1 (LATEST EDITION). ALL PLANTS SHALL MEET THE ADDITIONAL REQUIREMENTS SET FORTH BELOW AND IN WRITTEN SPECIFICATIONS AS APPLICABLE. ALL TREES AND SHRUBS SHALL HAVE BEEN GROWN AT A COMMERCIAL NURSERY WITHIN 200 MILES OF THE PROJECT SITE UNLESS OTHERWISE APPROVED BY OWNER OR LANDSCAPE ARCHITECT. THEY SHALL BE TYPICAL OF THEIR SPECIES OR VARIETY. THEY SHALL BE HEALTHY, SYMMETRICAL, EVENLY AND DENSELY BRANCHED, AND DENSELY FOLIATED WHEN IN LEAF. THEY SHALL BE FREE OF BARK INJURY, DISEASE, AND INSECT PESTS. ALL TREES SHALL HAVE A STRAIGHT TRUNK WITH A SINGLE MAIN LEADER UNLESS OTHERWISE CHARACTERISTIC OF THE SPECIES OR VARIETY. THE OWNER OR LANDSCAPE ARCHITECT WILL ALLOW SUBSTITUTIONS ONLY UPON WRITTEN APPROVAL. SIZES SHALL CONFORM TO THE MEASUREMENT SPECIFIED ON THE DRAWINGS. PLANTS LARGER THAN SPECIFIED MAY BE USED IF APPROVED, BUT THE USE OF SUCH PLANTS SHALL NOT INCREASE THE CONTRACT PRICE. ALL OVERSTORY TREES PLANTED ALONG PARKING AREAS, SIDEWALKS AND PEDESTRIAN ACCESSES SHALL NOT BRANCH BELOW 7' FEET IF THE TREE CALIPER IS 3" INCHES OR GREATER. ALL PLANT MATERIALS ARE SUBJECT TO INSPECTION AND ACCEPTANCE BY THE OWNER OR LANDSCAPE ARCHITECT AT THE NURSERY SOURCE. THE CONTRACTOR SHALL COORDINATE SOURCE VISITS WITH THE LANDSCAPE ARCHITECT AND SHALL ACCOMPANY THE OWNER AND/OR LANDSCAPE ARCHITECT FOR ALL INSPECTIONS. CERTIFICATES OF COMPLIANCE WITH SPECIFICATIONS ARE REQUIRED FOR ALL PLANTS.

ALL PLANT MATERIAL SHALL BE SUBJECT TO INSPECTION AND ACCEPTANCE BY THE OWNER OR LANDSCAPE ARCHITECT AT THE NURSERY SOURCE OR PLACE OF GROWTH. THE CONTRACTOR SHALL COORDINATE WITH THE LANDSCAPE ARCHITECT ON A SCHEDULE FOR SOURCE VISITS AND ACCOMPANY THE OWNER OR LANDSCAPE ARCHITECT FOR ALL SOURCE INSPECTIONS. CERTIFICATES OF COMPLIANCE ARE

PHOTOGRAPHIC REVIEW OF PLANT MATERIAL IS ACCEPTABLE IF APPROVED BY LANDSCAPE ARCHITECT. PHOTOGRAPHS MUST BE PROVIDED IN QUANTITY AND VARIETY TO ALLOW LANDSCAPE ARCHITECT SUFFICIENT INFORMATION TO MAKE A REASONABLE DETERMINATION AS TO THE PLANTS' QUALITY. OWNER AND LANDSCAPE ARCHITECT RESERVES THE RIGHT TO REJECT PLANT MATERIAL DELIVERED TO THE SITE BUT PREVIOUSLY ACCEPTED IF DAMAGED OR NOT PROPERLY MAINTAINED DURING THE DELIVERY PROCESS.

NS (UNLESS OTHERWISE	APPROVED BY THE OWNER OR LANI	DSCAPE ARCHITECT)
	SPRING FALL	
S AND SHRUBS	APRIL 1 TO JUNE 15 SEPT	EMBER 1 TO OCTOBER 15
S AND SHRUBS	APRIL 1 TO JUNE 15 SEPTI	EMBER 15 TO NOVEMBER 15
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	PER MANUFACTURERS RECOMMENDA	TIONS OR AS LISTED IN SEED MIX NOTES

SEEDING MIXTURES: REFER TO SEED MIX NOTES. SEEDED AREA SHALL BE ACCEPTED WHEN SEED AREA ACHIEVES 90% COVERAGE.

16. ALL SLOPES STEEPER THAN 3:1 RECEIVING A SEED MIX SHALL BE COVERED WITH AN EROSION CONTROL BLANKET OF STRAW FIBER AND BIODEGRADABLE OR PHOTODEGRADABLE NETTING.

17. UNLESS OTHERWISE NOTED IN DRAWING SET, NEW TREELINES SHALL EQUAL CLEARING AND GRUBBING LIMIT FOR CONSTRUCTION. 18. ALL DISTURBED AREAS NOT OTHERWISE DEVELOPED SHALL BE SEEDED WITH THE LAWN SEED MIX.

19. ALL SHADE TREE, BUFFER YARD AND OTHER LANDSCAPING REQUIRED BY LOCAL ORDINANCE OR ZONING SHALL BE PERPETUALLY MAINTAINED BY THE PROPERTY OWNER. ANY LANDSCAPING NEEDED TO MEET AN ORDINANCE OR ZONING REQUIREMENT THAT DIES, IS REMOVED, OR IS SEVERELY DAMAGED SHALL BE REPLACED BY THE CURRENT PROPERTY OWNER AS SOON AS IS PRACTICAL CONSIDERING GROWING SEASONS, WITH A MAXIMUM OF 150 DAYS.

![](_page_13_Picture_37.jpeg)

![](_page_13_Picture_38.jpeg)

LANDSCAPE NOTES AND DETAILS

heet No

![](_page_14_Picture_0.jpeg)

![](_page_14_Figure_2.jpeg)

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED

![](_page_14_Figure_7.jpeg)

![](_page_14_Figure_8.jpeg)

![](_page_15_Figure_0.jpeg)

021 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES

![](_page_16_Figure_0.jpeg)

NGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES

![](_page_16_Figure_3.jpeg)

## TYPICAL STORM SEWER TRENCH SECTION

N.T.S.

![](_page_16_Figure_5.jpeg)

![](_page_16_Figure_6.jpeg)

![](_page_16_Figure_7.jpeg)

![](_page_16_Figure_8.jpeg)

![](_page_16_Figure_9.jpeg)

![](_page_17_Figure_0.jpeg)

ENE	RG

N.T.S.

<b>BOLTON PLANNING AND ZONING</b>	COMMISSION,	BOLTON,	CI

DATE APPROVED DATE OF EXPIRATION

\_CHAIRMAN

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON

🗘 2021 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES.

# Y DISSIPATION TRENCH

# TYPICAL BIORETENTION BASIN DETAIL N.T.S.

- 5. REFER TO LANDSCAPING PLAN FOR PLANT AND SEED SCHEDULE FOR BIORETENTION BASIN.
- 4. THE OWNER IS RESPONSIBLE FOR MAINTENANCE OF BIORETENTION BASIN.
- 3. THE BIORETENTION BASIN WAS DESIGNED USING RECOMMENDED INFILTRATION RATE THROUGH THE PLANTING SOIL OF 5 IN/HR.
- 2. RECHARGE WILL BE PROVIDED BY BIORETENTION BASIN.
- RAINFALL.
- NOTES AND DESIGN REQUIREMENTS

BOTTOM OF CRUSHED STONE

(A)

656.50 FT

BIORETENTION BASIN #1

PONDED DEPTH PER STORM EVENT (FT)	1" WATER QUALITY STORM	100YR
BIORETENTION BASIN #1	656.16	661.91

657.50 FT

![](_page_17_Figure_17.jpeg)

# FOR PERMITTING PURPOSES ONLY NOT RELEASED FOR CONSTRUCTION

7. A RAISED UNDERDRAIN SYSTEM IS BEING UTILIZED TO CREATE AN INTERNAL WATER STORAGE ZONE WITHIN THE BIORETENTION SOIL MEDIA. THIS CONFIGURATION WILL ALLOW FOR A 6" DEPTH OF SUBMERGED BIORETENTION SOIL TO PROVIDE ENHANCED NITROGEN REMOVAL

FILTER FABRIC – GEOTEXTILE FABRIC SHALL MEET ASTM D-751 (PUNCTURE STRENGTH -125LB); ASTM D-1117 (MULLEN BURST STRENGTH-400LB); ASTM D-1682 (TENSILE STRENGTH-300 LB).

1. THE IMPOUNDED AREA CREATED ABOVE THE BIORETENTION SOIL MEDIA HAS BEEN SIZED TO HOLD WATER QUALITY VOLUME FROM FIRST 1"

TOP OF PEA GRAVEL/FILTER FABRIC/BOTTOM OF BIORETENTION SOIL MEDIA (C)	TOP OF BIORETENTION SOIL MEDIA (D)	WATER QUALITY VOLUME REQUIRED	WATER QUALITY VOLUME PROVIDED (E)	INTERNAL WATER STORAGE ZONE HEIGHT (F)
657.75 FT	660.50 FT	3,136 CF	661.87 FT (5,079 CF)	6"

![](_page_17_Picture_26.jpeg)

Architecture Engineering Environmentc Land Survevir

![](_page_18_Figure_0.jpeg)

![](_page_19_Figure_0.jpeg)

![](_page_20_Figure_0.jpeg)

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	$\underset{P_{12}}{\overset{P_{12}}{=}} 16. \frac{4.9}{5.1} \frac{5.1}{3.4} \frac{2.6}{2.7} \frac{2.7}{5.0} \frac{10.4}{10.4} \frac{12.9}{2.3} \frac{2.3}{0.2} \frac{0.1}{0.1} \frac{0.1}{0.1} \frac{0.1}{0.0} \frac{0.0}{0.0} \frac{0.1}{0.1} \frac{0.1}{0.$		0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	
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Luminaire Schedule		Calculation Summary			
Symbol Qty Label Arrangement Total Lamp Lumens	LLF Description	Label	CalcType U	Jnits Avg Max Min	Avg/Min Max/Min
→         3         HP-S         SINGLE         13632           →         4         HB         SINGLE         19188	0.900         LEDS-1210-S - Single Pole Mt 150W, Type 4, 5K, Shielded           0.950         LEDS-AL120 - Wall Mt, 150w, Type 4, 5K	Site Parking Lot	IIIuminance F	c         0.24         16.9         0.0           c         3.75         16.9         0.0	N.A. N.A. N.A. N.A.
6 G SINGLE 4740	0.950 LEDBG42W001B-5000K - Wall Pack, 42W, Full Cutoff, 5K				

# Notes:

Plan Notes:

Calculations at Ground Level (10' x 10' Grid Spacing). Refer to luminaire location summary for mounting heights of each fixture. Pole mounted fixtures include a 2ft concrete base. Mounting heights indicated on luminaire location summary is a total A.F.G. height.

# General Notes:

Due to changing lighting ordinances it is the contractors resposibility to submit the site photometrics & luminaire specs to the local inspector before ordering to ensure this plan complies with local lighting ordinances. This lighting design is based on information supplied by others. Changes in electrical supply, area geometry & objects within the lighted area may produce illumination values different from the predicted results shown on this layout. This layout is based on .IES files that were lab tested or computer generated, actual results may vary.

![](_page_21_Figure_6.jpeg)

Luminaire	Location Sur	mmary
LumNo	Label	Z
1	G	16.5
2	G	16.5
3	G	12
4	G	12
5	G	12
6	HB	16.5
7	HB	16.5
8	G	16.5
9	HB	16.5
10	HB	16.5
11	HP-S	17
12	HP-S	17
13	HP-S	17

4.4	
3.8	
3.1	
2.5	
1.9	
1.3	
0.63	
0	
lluminar Fc)	

![](_page_21_Picture_9.jpeg)

![](_page_21_Picture_10.jpeg)

![](_page_21_Picture_11.jpeg)

![](_page_21_Picture_12.jpeg)

![](_page_21_Picture_13.jpeg)

Pa		Drawn By: BMF, LC	# Date Comments	
age		Checked By:	Re	
1		Date:3/29/2021	evis	E
of 1			ion	EC
	Rolton CT 22222		IS	פ
		Scale:		

![](_page_22_Figure_0.jpeg)

![](_page_22_Figure_1.jpeg)

![](_page_22_Figure_2.jpeg)

![](_page_22_Figure_3.jpeg)

# PROPOSED REAR ELEVATION SCALE: 3/16"-1'-0"

4

Bolton, CT Retail Building Conceptual Scheme

![](_page_22_Figure_6.jpeg)

![](_page_22_Figure_7.jpeg)

	EXTERIO	R FINISH SCHEDU	ILE
TAG	MATERIAL/ MFG.	COLOR/ NO.	NOTES
(X1)	HARDIEE-PLANK LAP SIDING	COLOR: PEARL GRAY	PRE-FINISHED; 6" EXPOSURE
(X2)	HARDIEE-PLANK LAP SIDING	COLOR: NIGHT GRAY	PRE-FINISHED 6" EXPOSURE
(X3)	VEE BRICK	COLOR: TAVERN FLASH	
X4)	METAL COPING	COLOR: WHITE	PRE-FINISHED
X5	HARDIE-BOARD TRIM	COLOR: WHITE TO MATCH X4	COPING BY GC PRE-FINISHED
(X6)	METAL WALL PANEL	COLOR: CHARCOAL GRAY O.A.E.	PRE-FINISHED
X7	SURFACE APPLIED STOREFRONT	COLOR: DARK BRONZE	LIGHT GRAY SPANDREL
X7a	HARDIE-BOARD TRIM	COLOR: TO MATCH X1	PAINTED
(X8)	GUTTER & DOWNSPOUT	COLOR: DARK BRONZE	PRE-FINISHED
(X9)	EPDM ROOF	COLOR: DARK GRAY	PRE-FINISHED
X10	HARDIE-BOARD TRIM	COLOR: TO MATCH X2	PAINTED
X11	METAL COPING	COLOR: CHARCOAL GRAY	PRE-FINISHED
X12	INSULATED SLIDING ENTRY DOORS	COLOR: DARK BRONZE	PRE-FINISHED
X13	METAL DOOR & FRAME	COLOR: TO MATCH X2	PAINTED
X14	ARCH ASPHALT SHINGLES	COLOR: PEWTER GREY	GAF TIMBERLINE

SCALE: NOTED 03, May 2021

![](_page_22_Picture_12.jpeg)

Conceptual Elevations

0—

![](_page_22_Picture_14.jpeg)

Boston + Brockton 142 Crescent Street Brockton, MA 02302 508.583.5603 bkaarchitects.com

![](_page_23_Figure_0.jpeg)

![](_page_24_Picture_1.jpeg)

### Stormwater Management Narrative and Hydrologic Calculations Proposed Retail Development – 1100 Boston Turnpike – Bolton, CT May 5, 2021

This narrative has been prepared in support of a Permit Application by Garrett Homes, LLC to the Town of Bolton for the proposed retail development at 1100 Boston Turnpike. The property is approximately 1.85 acres in size and is currently an undeveloped parcel. The property is located on the northern side of Boston Turnpike and is roughly bordered by residential properties to the west and south and a dentist office on the previously subdivided parcel to the east. The site is bordered by undeveloped woodland and Bolton Lake to the north. The subject parcel described in this report is proposed to be subdivided from "Parcel 2" to the north.

### **Existing Site Conditions**

The project parcel is currently undeveloped, consisting entirely of lawn area. There are no formal stormwater management systems currently located on site. Stormwater from the subject property sheet flows untreated to the adjacent properties.

The site soil identified by the United States Department of Agriculture (USDA) Natural Resources Conservation Services (NRCS) is Woodbridge fine sandy loam, 3 to 8 percent slopes, Woodbridge fine sandy loam, 0 to 8 percent slopes, very stony, and Canton and Charlton fine sandy loams, 3 to 8 percent slopes. Per the USDA, the NRCS Hydrologic Soil Group rating for within the project area is C/D, C/D, and B respectively. For the Soil Group ratings of C/D, a Soil Group rating of C was assumed in order to be conservative in the change of curve number from grass to impervious.

### **Developed Site Conditions**

The proposed site improvements will include a 10,640 square foot retail building, paved parking areas, landscaped areas, pedestrian sidewalks, site utilities and lighting, and a stormwater management system.

The proposed stormwater management system will utilize a surface bioretention basin for stormwater quality treatment and peak flow mitigation of stormwater runoff generated by impervious surfaces eventually draining to the neighboring property to the east.

### **Stormwater Management – Existing Drainage Patterns**

The existing site drainage area that was analyzed totals 4.92 acres and is approximately 8% impervious.

Stormwater from the subject property sheet flows untreated to the adjacent properties. There is a ridge line that roughly bifurcates the site into two main drainage areas. The northern portion of the project parcel and neighboring properties sheet flow to the wetland to the northeast of the site (Design Point 1). The northern portion of the site consists of primarily of grassed surface cover

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![](_page_25_Picture_0.jpeg)

with some wooded and impervious surface cover. The southeastern portion of the project parcel sheet flows to the existing catch basins within the shared driveway to the east that drains to the stormwater management system located within the previously subdivided parcel to the east (Design Point 2). The southeastern portion of the site consists mainly of grassed area with some impervious area from the shared driveway.

### Stormwater Management – Proposed Drainage Patterns

The proposed site drainage area totals 4.92 acres and is approximately 28% impervious.

The same Design Points used in the existing conditions analysis have been retained for the proposed analysis. The site stormwater system will provide stormwater retention and quality improvements through the installation of a Bioretention Basin with a grass filter strip and a formalized street sweeping program for the impervious surfaces. These measures will treat the stormwater quality flow through structural means to provide water quality treatment in conformance with the State of Connecticut Water Quality Manual. The proposed stormwater management system has been designed to treat the runoff generated by the proposed development for a minimum 80% TSS removal as required in the CT Stormwater Quality Manual, retain and infiltrate the Water Quality Volume, and provide groundwater recharge.

As noted from town staff during a pre-application meeting, due to the site being in the lower reach of the watershed peak flow mitigation has not been deemed necessary to the wetland located northeast of the site (Design Point 1). Peak flow to the existing offsite drainage system on the parcel to the east (Design Point 2) will be matched in the 2, 10, and 100-year storms to ensure the proposed development will not negatively impact the existing neighboring drainage system, as seen in the peak flow rate comparison table below.

	Peak Flow Rate in Cubic Feet per Second (cfs)						
Drainage Area	2-yr 10-yr		100-yr				
Design Point 1	•	· · · · ·	•				
Wetland to Northeast							
Existing	2.6	6.3	12.9				
Proposed	2.0	4.7	9.3				
Percent Change	-23.1%	-25.4%	-27.9%				
Design Point 2							
Ex. CBs in Driveway							
Existing	0.9	1.8	3.3				
Proposed	0.9	1.8	3.2				
Percent Change	0.0%	0.0%	-3.0%				

![](_page_26_Picture_0.jpeg)

### Conclusion

The post-development peak discharge rates for the total developed site have been decreased or matched for all storm events. All post development stormwater will be discharged offsite to mimic existing drainage patterns. The proposed Bioretention Basin been designed to attenuate peak flows to Design Point 2 at the offsite drainage system, while providing water quality improvements. Though it was not necessary to match peak flows to Design Point 1, the flow has been mitigated by reducing the size of the contributing drainage area. The area removed from the drainage area to Design Point 1 now contributes to the proposed Bioretention Basin drainage area, which ultimately discharges to Design Point 2.

This letter has been prepared to compliment the submitted project plans and full Stormwater Management Report, as well as to represent the technical basis for the designs presented herein.

![](_page_27_Picture_0.jpeg)

### APPENDIX A

### DRAINAGE MAPS

ED-1 – Existing Hydrology Mapping PD-1 – Proposed Hydrology Mapping

### EXISTING HYDROLOGY INFORMATION

				PERCENT		TIME OF
		IMPERVIOUS	PERVIOUS	IMPERVIOUS		CONCETRATIONS
DRAINAGEA AREA	TOTAL AREA (S.F.)	AREA (S.F.)	AREA (S.F.)	(%)	CN	(MIN.)
EDA-10	185,210	12,135	173,075	6.6%	72	14.5
EDA-20	29,230	4,605	24,625	15.8%	81	25.3

### HYDROLOGY LEGEND

![](_page_28_Figure_3.jpeg)

PROPERTY LINE DRAINAGE AREA BOUNDARY TIME OF CONCENTRATION FLOW PATH SOIL TYPE BOUNDARY SOIL TYPE DESIGNATION

![](_page_28_Picture_5.jpeg)

GRAPHIC SCALE 80 40 0 80 SCALE IN FEET

![](_page_28_Picture_7.jpeg)

### ARCHITECTURE ENGINEERING ENVIRONMENTAL LAND SURVEYING Companies

100 Constitution Plaza, 10th Floor Hartford, CT 06103 (860) 249-2200 (860) 249-2400 Fax

PROPOSED RETAIL DEVELOPMENT 1100 BOSTON TURNPIKE BOLTON, CONNECTICUT

Reviewed Scale Project No. Date CAD File

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1"=80' 2002032 04/02/2021 ED200203201

### PROPOSED HYDROLOGY INFORMATION

				PERCENT		TIME OF
		IMPERVIOUS	PERVIOUS	IMPERVIOUS		CONCETRATIONS
DRAINAGEA AREA	TOTAL AREA (S.F.)	AREA (S.F.)	AREA (S.F.)	(%)	CN	(MIN.)
PDA-101	127,015	15,295	111,720	12.0%	74	25.0
PDA-201	22,250	5,970	16,280	26.8%	83	9.8
PDA-202	54,405	27,695	26,710	50.9%	85	16.3
PDA-203	10,770	10,770	0	100.0%	98	5.0

# HYDROLOGY LEGEND

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PROPERTY LINE DRAINAGE AREA BOUNDARY TIME OF CONCENTRATION FLOW PATH SOIL TYPE BOUNDARY SOIL TYPE DESIGNATION

![](_page_29_Picture_4.jpeg)

GRAPHIC SCALE 80 40 0 80 SCALE IN FEET

![](_page_29_Picture_6.jpeg)

![](_page_29_Picture_7.jpeg)

100 Constitution Plaza, 10th Floor Hartford, CT 06103 (860) 249-2200 (860) 249-2400 Fax

1100 BOSTON TURNPIKE BOLTON, CONNECTICUT

Date CAD File

PD200203201

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![](_page_30_Picture_0.jpeg)

### APPENDIX B

### PRE-DEVELOPMENT HYDROLOGY

![](_page_31_Figure_0.jpeg)

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-10: Area to Wetland to Runoff Area=185,210 sf 6.55% Impervious Runoff Depth>0.99" Flow Length=136' Slope=0.0145 '/' Tc=25.0 min CN=72 Runoff=2.57 cfs 0.350 af

Subcatchment EDA-20: Area to Ex. CBs in Runoff Area=29,230 sf 15.75% Impervious Runoff Depth>1.55" Flow Length=169' Tc=14.3 min CN=81 Runoff=0.92 cfs 0.087 af

**Reach DP-1: Wetland to Northeast** 

Inflow=2.57 cfs 0.350 af Outflow=2.57 cfs 0.350 af

Reach DP-2: Ex. CBs in Driveway

Inflow=0.92 cfs 0.087 af Outflow=0.92 cfs 0.087 af

Total Runoff Area = 4.923 ac Runoff Volume = 0.437 af Average Runoff Depth = 1.06" 92.19% Pervious = 4.539 ac 7.81% Impervious = 0.384 ac

C-DAT-2002032-EXISTING HYDROLOGY	CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"
Prepared by {enter your company name here}	Printed 5/4/2021
HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD \$	Software Solutions LLC Page 3

### Summary for Subcatchment EDA-10: Area to Wetland to the Northeast

Runoff = 2.57 cfs @ 12.31 hrs, Volume= 0.350 af, Depth> 0.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

Ar	ea (sf)	CN	Description						
1:	34,225	69	50-75% Gra	)-75% Grass cover, Fair, HSG B					
	15,340	79	50-75% Gra	)-75% Grass cover, Fair, HSG C					
	2,445	60	Woods, Fai	/oods, Fair, HSG B					
	21,065	73	Woods, Fai	Voods, Fair, HSG C					
	12,135	98	Paved park	aved parking, HSG B					
	0	98	Paved park	Paved parking, HSG C					
18	85,210	72	Weighted Average						
17	73,075		93.45% Pervious Area						
	12,135		6.55% Impe	ervious Area	а				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)					
24.0	100	0.0145	5 0.07		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.31"				
1.0	36	0.0145	5 0.60		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
0 - 0	100	<b>—</b> · ·							

25.0 136 Total

### Subcatchment EDA-10: Area to Wetland to the Northeast

![](_page_33_Figure_7.jpeg)

C-DAT-2002032-EXISTING HYDROLOGY	CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"
Prepared by {enter your company name here}	Printed 5/4/2021
HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD \$	Software Solutions LLC Page 4

### Summary for Subcatchment EDA-20: Area to Ex. CBs in Driveway

Runoff = 0.92 cfs @ 12.15 hrs, Volume= 0.087 af, Depth> 1.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

A	rea (sf)	CN	CN Description				
	2,335	69 50-75% Grass cover, Fair, HSG B					
	22,290	79 50-75% Grass cover, Fair, HSG C					
	0	60	Woods, Fai	ir, HSG B			
	0	73 Woods, Fair, HSG C					
	185	98	98 Paved parking, HSG B				
	4,420	98	Paved park	ing, HSG C			
	29,230	81	Weighted A	verage			
	24,625	84.25% Pervious Area					
	4,605	15.75% Impervious Area					
Tc	Length	Slope	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft	:) (ft/sec)	(cfs)			
13.9	100	0.008	0.12		Sheet Flow,		
					Grass: Short n= 0.150 P2= 3.31"		
0.2	16	0.0284	4 1.18		Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
0.1	15	0.010	0 2.03		Shallow Concentrated Flow,		
					Paved Kv= 20.3 fps		
0.1	38	0.018	5 7.16	8.79	Pipe Channel,		
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'		
					n= 0.013 Corrugated PE, smooth interior		
14.3	169	Total					

![](_page_35_Figure_1.jpeg)

### Subcatchment EDA-20: Area to Ex. CBs in Driveway
#### Summary for Reach DP-1: Wetland to Northeast

Inflow A	Area	=	4.252 ac,	6.55% Impervious,	Inflow Depth > 0	.99" for 2-yr event
Inflow		=	2.57 cfs @	12.31 hrs, Volume	= 0.350 af	-
Outflow	/	=	2.57 cfs @	12.31 hrs, Volume	= 0.350 af	, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



## Reach DP-1: Wetland to Northeast

#### Summary for Reach DP-2: Ex. CBs in Driveway

Inflow A	Area =	0.671 ac,	15.75% Impervious,	Inflow Depth > 1.	55" for 2-yr event
Inflow	=	0.92 cfs @	) 12.15 hrs, Volume	= 0.087 af	
Outflow	/ =	0.92 cfs @	) 12.15 hrs, Volume	= 0.087 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



## Reach DP-2: Ex. CBs in Driveway

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-10: Area to Wetland to Runoff Area=185,210 sf 6.55% Impervious Runoff Depth>2.24" Flow Length=136' Slope=0.0145 '/' Tc=25.0 min CN=72 Runoff=6.27 cfs 0.794 af

Subcatchment EDA-20: Area to Ex. CBs in Runoff Area=29,230 sf 15.75% Impervious Runoff Depth>3.04" Flow Length=169' Tc=14.3 min CN=81 Runoff=1.82 cfs 0.170 af

**Reach DP-1: Wetland to Northeast** 

Inflow=6.27 cfs 0.794 af Outflow=6.27 cfs 0.794 af

Reach DP-2: Ex. CBs in Driveway

Inflow=1.82 cfs 0.170 af Outflow=1.82 cfs 0.170 af

Total Runoff Area = 4.923 ac Runoff Volume = 0.964 af Average Runoff Depth = 2.35" 92.19% Pervious = 4.539 ac 7.81% Impervious = 0.384 ac

#### Summary for Subcatchment EDA-10: Area to Wetland to the Northeast

Runoff = 6.27 cfs @ 12.30 hrs, Volume= 0.794 af, Depth> 2.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

Ar	rea (sf)	CN [	Description		
1	34,225	69 5	50-75% Gra	ass cover, F	Fair, HSG B
	15,340	79 5	50-75% Gra	ass cover, F	Fair, HSG C
	2,445	60 \	Noods, Fai	r, HSG B	
	21,065	73 \	Noods, Fai	r, HSG C	
	12,135	98 F	Paved park	ing, HSG B	
	0	98 F	Paved park	ing, HSG C	
1	85,210	72 \	Neighted A	verage	
1	73,075	ę	93.45% Per	rvious Area	
	12,135	6	6.55% Impe	ervious Area	а
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
24.0	100	0.0145	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.31"
1.0	36	0.0145	0.60		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps

25.0 136 Total

#### Subcatchment EDA-10: Area to Wetland to the Northeast



#### Summary for Subcatchment EDA-20: Area to Ex. CBs in Driveway

Runoff = 1.82 cfs @ 12.15 hrs, Volume= 0.170 af, Depth> 3.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

A	rea (sf)	CN	Description		
	2,335	69	50-75% Gra	ass cover, F	Fair, HSG B
	22,290	79	50-75% Gra	ass cover, F	Fair, HSG C
	0	60	Woods, Fai	r, HSG B	
	0	73	Woods, Fai	r, HSG C	
	185	98	Paved park	ing, HSG B	
	4,420	98	Paved park	ing, HSG C	
	29,230	81	Weighted A	verage	
	24,625		84.25% Pei	rvious Area	
	4,605	15.75% Impervious Are			ea
Tc	Length	Slop	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft	:) (ft/sec)	(cfs)	
13.9	100	0.008	0.12		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.31"
0.2	16	0.028	4 1.18		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.1	15	0.010	0 2.03		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.1	38	0.018	5 7.16	8.79	Pipe Channel,
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Corrugated PE, smooth interior
14.3	169	Total			



## Subcatchment EDA-20: Area to Ex. CBs in Driveway

#### Summary for Reach DP-1: Wetland to Northeast

Inflow A	٩rea	=	4.252 ac,	6.55% Impervious,	Inflow Depth > 2	.24" for 10-yr event
Inflow		=	6.27 cfs @	12.30 hrs, Volume	= 0.794 af	-
Outflow	/	=	6.27 cfs @	12.30 hrs, Volume	= 0.794 af	, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



## Reach DP-1: Wetland to Northeast

#### Summary for Reach DP-2: Ex. CBs in Driveway

Inflow A	Area =	0.671 ac, 15.75% Impervious, In	nflow Depth > 3.04" for 10-yr event	
Inflow	=	1.82 cfs @ 12.15 hrs, Volume=	0.170 af	
Outflow	/ =	1.82 cfs @ 12.15 hrs, Volume=	0.170 af, Atten= 0%, Lag= 0.0 m	in

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



## Reach DP-2: Ex. CBs in Driveway

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-10: Area to Wetland to Runoff Area=185,210 sf 6.55% Impervious Runoff Depth>4.57" Flow Length=136' Slope=0.0145 '/' Tc=25.0 min CN=72 Runoff=12.93 cfs 1.620 af

Subcatchment EDA-20: Area to Ex. CBs in Runoff Area=29,230 sf 15.75% Impervious Runoff Depth>5.63" Flow Length=169' Tc=14.3 min CN=81 Runoff=3.30 cfs 0.315 af

**Reach DP-1: Wetland to Northeast** 

Inflow=12.93 cfs 1.620 af Outflow=12.93 cfs 1.620 af

Reach DP-2: Ex. CBs in Driveway

Inflow=3.30 cfs 0.315 af Outflow=3.30 cfs 0.315 af

Total Runoff Area = 4.923 ac Runoff Volume = 1.934 af Average Runoff Depth = 4.72" 92.19% Pervious = 4.539 ac 7.81% Impervious = 0.384 ac

#### Summary for Subcatchment EDA-10: Area to Wetland to the Northeast

Runoff = 12.93 cfs @ 12.29 hrs, Volume= 1.620 af, Depth> 4.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

Area (sf)	CN	Description	l	
134,225	69	50-75% Gr	ass cover, l	Fair, HSG B
15,340	79	50-75% Gra	ass cover, l	Fair, HSG C
2,445	60	Woods, Fa	ir, HSG B	
21,065	73	Woods, Fa	ir, HSG C	
12,135	98	Paved park	ing, HSG B	6
0	98	Paved park	ting, HSG C	
185,210	72	Weighted A	Verage	
173,075		93.45% Pe	rvious Area	
12,135		6.55% Imp	ervious Are	а
Tc Length	Slop	e Velocity	Capacity	Description
(min) (feet)	(ft/f	t) (ft/sec)	(cfs)	
24.0 100	0.014	5 0.07		Sheet Flow,
				Woods: Light underbrush n= 0.400 P2= 3.31"
1.0 36	0.014	5 0.60		Shallow Concentrated Flow,
				Woodland Kv= 5.0 fps
	<b>—</b> · ·			

25.0 136 Total

#### Subcatchment EDA-10: Area to Wetland to the Northeast



#### Summary for Subcatchment EDA-20: Area to Ex. CBs in Driveway

Runoff = 3.30 cfs @ 12.15 hrs, Volume= 0.315 af, Depth> 5.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

A	rea (sf)	CN	Description					
	2,335	69	69 50-75% Grass cover, Fair, HSG B					
	22,290	79	9 50-75% Grass cover, Fair, HSG C					
	0	60	Woods, Fai	r, HSG B				
	0	73	Woods, Fai	r, HSG C				
	185	98	Paved parking, HSG B					
	4,420	98						
	29,230	81	Weighted A	verage				
	24,625		84.25% Pe	rvious Area				
	4,605	15.75% Impervious Are		pervious Are	ea			
Тс	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)				
13.9	100	0.0080	0.12		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.31"			
0.2	16	0.0284	4 1.18		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.1	15	0.0100	2.03		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
0.1	38	0.018	5 7.16	8.79	Pipe Channel,			
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'			
					n= 0.013 Corrugated PE, smooth interior			
14.3	169	Total						



## Subcatchment EDA-20: Area to Ex. CBs in Driveway

#### Summary for Reach DP-1: Wetland to Northeast

Inflow /	Area	a =	4.252 ac,	6.55% Impervious,	Inflow Depth > 4	4.57"	for 100-yr event
Inflow		=	12.93 cfs @	12.29 hrs, Volume	= 1.620 a	af	-
Outflov	v	=	12.93 cfs @	12.29 hrs, Volume	= 1.620 a	af, Attei	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



## Reach DP-1: Wetland to Northeast

#### Summary for Reach DP-2: Ex. CBs in Driveway

Inflow A	Area =	=	0.671 ac,	15.75% Impe	ervious,	Inflow Dep	oth > 5.	.63" for	100-yr event
Inflow	=		3.30 cfs @	12.15 hrs,	Volume	= (	0.315 af		
Outflow	/ =		3.30 cfs @	12.15 hrs,	Volume	= (	0.315 af,	, Atten= (	0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



## Reach DP-2: Ex. CBs in Driveway



## APPENDIX C

# POST-DEVELOPMENT HYDROLOGY



# C-DAT-2002032-PROPOSED HYDROLOGCT-Coventry-2002032 24-hr S1 2-yrRainfall=3.31"Prepared by {enter your company name here}Printed 5/4/2021HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD Software Solutions LLCPage 2

Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method Subcatchment PDA-101: Area to Wetland Runoff Area=127,015 sf 12.04% Impervious Runoff Depth=1.11" Flow Length=136' Slope=0.0145 '/' Tc=25.0 min CN=74 Runoff=2.03 cfs 0.270 af Subcatchment PDA-201: Area to Ex. CBs Runoff Area=22,250 sf 26.83% Impervious Runoff Depth=1.70" Flow Length=161' Tc=9.8 min CN=83 Runoff=0.94 cfs 0.072 af Runoff Area=54,405 sf 50.91% Impervious Runoff Depth=1.85" Subcatchment PDA-202: Area to Flow Length=250' Slope=0.0100 '/' Tc=16.3 min CN=85 Runoff=1.94 cfs 0.193 af Runoff Area=10.770 sf 100.00% Impervious Runoff Depth=3.08" Subcatchment PDA-203: Building Area Tc=5.0 min CN=98 Runoff=0.96 cfs 0.063 af Avg. Flow Depth=0.31' Max Vel=1.73 fps Inflow=0.96 cfs 0.063 af Reach 1R: Swale to Basin n=0.030 L=370.0' S=0.0105 '/' Capacity=9.44 cfs Outflow=0.81 cfs 0.063 af Reach DP-1: Wetland to Northeast Inflow=2.03 cfs 0.270 af Outflow=2.03 cfs 0.270 af Reach DP-2: Ex. CBs in Driveway Inflow=0.94 cfs 0.072 af Outflow=0.94 cfs 0.072 af

Pond Pond #1: Bioretention BasinPeak Elev=659.88' Storage=5,907 cfInflow=2.64 cfs0.256 afDiscarded=0.10 cfs0.256 afPrimary=0.00 cfs0.000 afOutflow=0.10 cfs0.256 af

Total Runoff Area = 4.923 ac Runoff Volume = 0.598 af Average Runoff Depth = 1.46" 72.15% Pervious = 3.552 ac 27.85% Impervious = 1.371 ac

#### Summary for Subcatchment PDA-101: Area to Wetland to the Northeast

Runoff = 2.03 cfs @ 12.31 hrs, Volume= 0.270 af, Depth= 1.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

A	vrea (sf)	CN	Description		
	78,455	69	50-75% Gra	ass cover, F	Fair, HSG B
	10,060	79	50-75% Gra	ass cover, F	Fair, HSG C
	2,445	60	Woods, Fai	ir, HSG B	
	20,760	73	Woods, Fai	ir, HSG C	
	13,830	98	Paved park	ing, HSG B	
	1,465 98 Paved parking, HSG C				
	127,015	74	Weighted A	verage	
	111,720	1	87.96% Pei	rvious Area	
	15,295		12.04% Imp	pervious Are	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
24.0	100	0.0145	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.31"
1.0	36	0.0145	0.60		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps

25.0 136 Total

#### Subcatchment PDA-101: Area to Wetland to the Northeast



#### Summary for Subcatchment PDA-201: Area to Ex. CBs in Driveway

Runoff = 0.94 cfs @ 12.08 hrs, Volume= 0.072 af, Depth= 1.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

A	rea (sf)	CN I	Description					
	2,580	69	69 50-75% Grass cover, Fair, HSG B					
	13,700	79	79 50-75% Grass cover, Fair, HSG C					
	0	60	60 Woods, Fair, HSG B					
	0	73	73 Woods, Fair, HSG C					
	910	98 I	98 Paved parking, HSG B					
	5,060	98 I	98 Paved parking, HSG C					
	22,250	83	Neighted A	verage				
	16,280	-	73.17% Pei	vious Area				
	5,970		26.83% Imp	pervious Are	ea			
-			N / 1 · · ·	0				
	Length	Slope	Velocity	Capacity	Description			
(min)	(teet)	(π/π)	(IT/Sec)	(CIS)				
9.3	100	0.0220	0.18		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.31"			
0.4	23	0.0174	0.92		Shallow Concentrated Flow,			
					Short Grass Pasture Ky= 7 () tos			
<b>•</b> • •	~ ~		- 10	o <b>-</b> o				
0.1	38	0.0185	7.16	8.79	Pipe Channel,			
0.1	38	0.0185	7.16	8.79	Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'			
0.1	38	0.0185	7.16	8.79	Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Corrugated PE, smooth interior			



#### Subcatchment PDA-201: Area to Ex. CBs in Driveway

#### Summary for Subcatchment PDA-202: Area to Bioretention Basin

Runoff = 1.94 cfs @ 12.17 hrs, Volume= 0.193 af, Depth= 1.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

_	A	rea (sf)	CN	Description		
		20,135	69	50-75% Gra	ass cover, F	Fair, HSG B
		6,575	79	50-75% Gra	ass cover, F	Fair, HSG C
		0	60	Woods, Fai	ir, HSG B	
		0	73	Woods, Fai	ir, HSG C	
		22,195	98	Paved park	ing, HSG B	
_		5,500	98	Paved park	ing, HSG C	
		54,405	85	Weighted A	verage	
		26,710		49.09% Pe	rvious Area	
		27,695		50.91% Imp	pervious Are	ea
	Тс	Length	Slope	e Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	) (ft/sec)	(cfs)	
	12.7	100	0.0100	0.13		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.31"
	3.6	150	0.0100	0.70		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps

16.3 250 Total

#### Subcatchment PDA-202: Area to Bioretention Basin



C-DAT-2002032-PROPOSED HYDROLOG	CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"
Prepared by {enter your company name here}	Printed 5/4/2021
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#### Summary for Subcatchment PDA-203: Building Area

Runoff = 0.96 cfs @ 12.03 hrs, Volume= 0.063 af, Depth= 3.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

Area (sf)	CN	Description							
0	69	50-75% Gra	50-75% Grass cover, Fair, HSG B						
0	79	50-75% Gra	50-75% Grass cover, Fair, HSG C						
0	60	Woods, Fai	r, HSG B						
0	73	Woods, Fai	r, HSG C						
10,770	98	Paved park	ing, HSG B						
0	98	Paved park	ing, HSG C	,					
10,770	98	Weighted A	verage						
10,770		100.00% Im	pervious A	rea					
Tc Length (min) (feet)	Slor (ft/	be Velocity ft) (ft/sec)	Capacity (cfs)	Description					
5.0				Direct Entry,					

#### Subcatchment PDA-203: Building Area



C-DAT-2002032-PROPOSED HYDROLOGCT-Coventry-2002032 24-hr S1 2-yrRainfall=3.31"Prepared by {enter your company name here}Printed 5/4/2021HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD Software Solutions LLCPage 8

#### Summary for Reach 1R: Swale to Basin



#### Summary for Reach DP-1: Wetland to Northeast

Inflow Area	a =	2.916 ac, 1	2.04% Imper	rvious, Inflow [	Depth = 1.11"	for 2-yr event
Inflow	=	2.03 cfs @	12.31 hrs, \	/olume=	0.270 af	-
Outflow	=	2.03 cfs @	12.31 hrs, \	/olume=	0.270 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



## **Reach DP-1: Wetland to Northeast**

#### Summary for Reach DP-2: Ex. CBs in Driveway

Inflow Are	ea =	2.007 ac, 50.83%	6 Impervious, Inflov	v Depth = 0.43"	for 2-yr event
Inflow	=	0.94 cfs @ 12.08	3 hrs, Volume=	0.072 af	
Outflow	=	0.94 cfs @ 12.08	3 hrs, Volume=	0.072 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



## Reach DP-2: Ex. CBs in Driveway

#### Summary for Pond Pond #1: Bioretention Basin

Inflow Area	a =	1.496 ac, 5	9.02% Imp	ervious, li	nflow Depth =	2.05"	for 2-yr	event
Inflow	=	2.64 cfs @	12.14 hrs,	Volume=	0.256	af		
Outflow	=	0.10 cfs @	10.54 hrs,	Volume=	0.256	af, Atte	en= 96%,	Lag= 0.0 min
Discarded	=	0.10 cfs @	10.54 hrs,	Volume=	0.256	af		
Primary	=	0.00 cfs @	0.00 hrs,	Volume=	0.000	af		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 659.88' @ 17.72 hrs Surf.Area= 4,366 sf Storage= 5,907 cf

Plug-Flow detention time= 578.0 min calculated for 0.256 af (100% of inflow) Center-of-Mass det. time= 578.0 min (1,412.2 - 834.2)

Volume	Invert	Avail.Sto	orage Stora	age Description	
#1	656.50'	6,9	86 cf <b>Cus</b> t 17,4	t <b>om Stage Data (Pr</b> 64 cf Overall x 40.0	ismatic) Listed below (Recalc) 0% Voids
#2	660.50'	15,4	65 cf Cust	tom Stage Data (Pr	ismatic) Listed below (Recalc) -Impervious
		22,4	50 cf Tota	l Available Storage	
Elevatio	n Si	urf.Area	Inc.Store	e Cum.Store	
(fee	t)	(sq-ft)	(cubic-feet	) (cubic-feet)	
656.5	0	4,366	(	) 0	
660.5	0	4,366	17,464	17,464	
Elevatio	n Si	urf.Area	Inc.Store	e Cum.Store	
(fee	t)	(sq-ft)	(cubic-feet	) (cubic-feet)	
660.5	0	4,366	(	) 0	
661.0	0	4,888	2,314	2,314	
662.0	0	6,042	5,465	5 7,779	
663.0	0	7,775	6,909	9 14,687	
663.1	0	7,775	778	3 15,465	
Device	Routing	Invert	Outlet Dev	vices	
#1	Discarded	656.50'	1.000 in/h	r Exfiltration over S	Surface area
#2	Primary	658.25'	12.0" Roi	und Culvert	
#3	Device 2	661.87'	L= 69.0' Inlet / Out n= 0.013 <b>24.0'' x 36</b> Limited to	CPP, end-section c let Invert= 658.25' / Corrugated PE, sm <b>.0" Horiz. Orifice/G</b> weir flow at low hea	onforming to fill, Ke= 0.500 657.90' S= 0.0051 '/' Cc= 0.900 ooth interior, Flow Area= 0.79 sf firate C= 0.600 ads

**Discarded OutFlow** Max=0.10 cfs @ 10.54 hrs HW=656.57' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.10 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=656.50' (Free Discharge)

**3=Orifice/Grate** (Controls 0.00 cfs)



## Pond Pond #1: Bioretention Basin

# C-DAT-2002032-PROPOSED HYDROLOCT-Coventry-2002032 24-hr S1 10-yrRainfall=5.08"Prepared by {enter your company name here}Printed 5/4/2021HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD Software Solutions LLCPage 13

Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-101: Area to Wetland	Runoff Area=127,015 sf	5.0 min CN=74	vious Runoff De	oth=2.43"
Flow Length=136'	Slope=0.0145 '/' Tc=25		Runoff=4.67 cfs	0.590 af
Subcatchment PDA-201: Area to Ex. CBs	Runoff Area=22,250 sf	26.83% Imperv	/ious Runoff De	oth=3.25"
	Flow Length=161' Tc=9	9.8 min CN=83	Runoff=1.79 cfs	0.138 af
Subcatchment PDA-202: Area to	Runoff Area=54,405 sf	50.91% Imperv	vious Runoff De	oth=3.44"
Flow Length=250'	Slope=0.0100 '/' Tc=16	3.3 min CN=85	Runoff=3.57 cfs	0.358 af
Subcatchment PDA-203: Building Area	Runoff Area=10,770 sf	100.00% Imperv	vious Runoff De	oth=4.84"
	Tc=5	5.0 min CN=98	Runoff=1.47 cfs	0.100 af
Reach 1R: Swale to Basin n=0.030 L=	vg. Flow Depth=0.39' ۸	/lax Vel=1.98 fps	Inflow=1.47 cfs	0.100 af
	370.0' S=0.0105 '/' Ca	pacity=9.44 cfs	Outflow=1.28 cfs	0.100 af
Reach DP-1: Wetland to Northeast			Inflow=4.67 cfs Outflow=4.67 cfs	0.590 af 0.590 af
Reach DP-2: Ex. CBs in Driveway			Inflow=1.79 cfs Outflow=1.79 cfs	0.138 af 0.138 af
Pond Pond #1: Bioretention Basin	Peak Elev=661.76' St	orage=13,330 cf	Inflow=4.60 cfs	0.458 af
Discarded=0.10 c	fs 0.458 af Primary=0.0	00 cfs 0.000 af	Outflow=0.10 cfs	0.458 af

Total Runoff Area = 4.923 ac Runoff Volume = 1.186 af Average Runoff Depth = 2.89" 72.15% Pervious = 3.552 ac 27.85% Impervious = 1.371 ac

#### Summary for Subcatchment PDA-101: Area to Wetland to the Northeast

Runoff = 4.67 cfs @ 12.30 hrs, Volume= 0.590 af, Depth= 2.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

/	Area (sf)	CN I	Description						
	78,455	69 5	50-75% Grass cover, Fair, HSG B						
	10,060	79 క	50-75% Grass cover, Fair, HSG C						
	2,445	60 V	Woods, Fair, HSG B						
	20,760	73 \	Noods, Fai	r, HSG C					
	13,830	98 I	Paved park	ing, HSG B					
	1,465	98 I	Paved park	ing, HSG C					
	127,015	74 \	Neighted A	verage					
	111,720	8	37.96% Pei	rvious Area					
	15,295		12.04% Imp	pervious Are	ea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
24.0	100	0.0145	0.07		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.31"				
1.0	36	0.0145	0.60		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				

25.0 136 Total

#### Subcatchment PDA-101: Area to Wetland to the Northeast



#### Summary for Subcatchment PDA-201: Area to Ex. CBs in Driveway

Runoff = 1.79 cfs @ 12.08 hrs, Volume= 0.138 af, Depth= 3.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

A	rea (sf)	CN	Description							
	2,580	69	69 50-75% Grass cover, Fair, HSG B							
	13,700	79	79 50-75% Grass cover, Fair, HSG C							
	0	60	0 Woods, Fair, HSG B							
	0	73	Woods, Fair, HSG C							
	910	98	Paved park	ing, HSG B						
	5,060	98	Paved park	ing, HSG C						
	22,250	83	Weighted A	verage						
	16,280		73.17% Pei	vious Area						
	5,970		26.83% Imp	pervious Are	ea					
_				_						
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
9.3	100	0 0220	0 1 8							
		0.0220	0.10		Sneet Flow,					
		0.0220	0.10		Grass: Short n= 0.150 P2= 3.31"					
0.4	23	0.0174	0.10		Grass: Short n= 0.150 P2= 3.31" Shallow Concentrated Flow,					
0.4	23	0.0174	0.92		Grass: Short n= 0.150 P2= 3.31" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps					
0.4 0.1	23 38	0.0174	0.92 7.16	8.79	Grass: Short n= 0.150 P2= 3.31" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps Pipe Channel,					
0.4 0.1	23 38	0.0174	0.92	8.79	Sheet Flow, Grass: Short n= 0.150 P2= 3.31" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'					
0.4	23 38	0.0174	0.10	8.79	Sheet Flow, Grass: Short n= 0.150 P2= 3.31" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Corrugated PE, smooth interior					



#### Subcatchment PDA-201: Area to Ex. CBs in Driveway

#### Summary for Subcatchment PDA-202: Area to Bioretention Basin

Runoff = 3.57 cfs @ 12.17 hrs, Volume= 0.358 af, Depth= 3.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

A	rea (sf)	CN I	Description						
	20,135	69 క	50-75% Grass cover, Fair, HSG B						
	6,575	79 క	50-75% Gra	ass cover, F	Fair, HSG C				
	0	60 \	Noods, Fai	ir, HSG B					
	0	73 \	Noods, Fai	ir, HSG C					
	22,195	98 I	Paved park	ing, HSG B					
	5,500	98 I	Paved park	ing, HSG C					
	54,405	85 V	Neighted A	verage					
	26,710	4	19.09% Pe	rvious Area					
	27,695	Ę	50.91% Imp	pervious Are	ea				
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
12.7	100	0.0100	0.13		Sheet Flow,				
					Grass: Short n= 0.150 P2= 3.31"				
3.6	150	0.0100	0.70		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				

16.3 250 Total

#### Subcatchment PDA-202: Area to Bioretention Basin



#### Summary for Subcatchment PDA-203: Building Area

Runoff = 1.47 cfs @ 12.03 hrs, Volume= 0.100 af, Depth= 4.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

/	Area (sf)	CN	Description					
	0	69	50-75% Grass cover, Fair, HSG B					
	0	79	50-75% Grass cover, Fair, HSG C					
	0	60	Woods, Fair, I	HSG B				
	0	73	Woods, Fair, I	HSG C				
	10,770	98	Paved parking	j, HSG B				
	0	98	Paved parking	j, HSG C				
	10,770	98	Weighted Ave	erage				
	10,770		100.00% Impe	ervious A	rea			
Тс	: Length	Slop	e Velocity C	Capacity	Description			
(min)	(feet)	(ft/	t) (ft/sec)	(cfs)				
5.0					Direct Entry,			

#### Subcatchment PDA-203: Building Area



C-DAT-2002032-PROPOSED HYDROLOCT-Coventry-2002032 24-hr S1 10-yrRainfall=5.08"Prepared by {enter your company name here}Printed 5/4/2021HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD Software Solutions LLCPage 19

#### Summary for Reach 1R: Swale to Basin



#### Summary for Reach DP-1: Wetland to Northeast

Inflow Are	a =	2.916 ac, 1	2.04% Impervious,	Inflow Depth = 2	.43" for 10-yr event
Inflow	=	4.67 cfs @	12.30 hrs, Volume	= 0.590 af	
Outflow	=	4.67 cfs @	12.30 hrs, Volume	= 0.590 af	, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



## **Reach DP-1: Wetland to Northeast**

#### Summary for Reach DP-2: Ex. CBs in Driveway

Inflow Ar	ea =	2.007 ac, 50.83% Imp	ervious, Inflow	/ Depth = 0.83"	for 10-yr event
Inflow	=	1.79 cfs @ 12.08 hrs,	Volume=	0.138 af	
Outflow	=	1.79 cfs @ 12.08 hrs,	Volume=	0.138 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



## Reach DP-2: Ex. CBs in Driveway
## Summary for Pond Pond #1: Bioretention Basin

Inflow Area	=	1.496 ac, 5	9.02% Impervious,	Inflow Depth =	3.67" for	10-yr event
Inflow	=	4.60 cfs @	12.14 hrs, Volume	= 0.458 a	af	
Outflow	=	0.10 cfs @	8.55 hrs, Volume	= 0.458 a	af, Atten=	98%, Lag= 0.0 min
Discarded	=	0.10 cfs @	8.55 hrs, Volume	= 0.458 a	af	-
Primary	=	0.00 cfs @	0.00 hrs, Volume	= 0.000 a	af	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 661.76' @ 24.04 hrs Surf.Area= 4,366 sf Storage= 13,330 cf

Plug-Flow detention time= 1,190.4 min calculated for 0.458 af (100% of inflow) Center-of-Mass det. time= 1,190.4 min (2,006.8 - 816.4)

Invert	Avail.Sto	rage Stora	ge Description	
656.50'	6,9	86 cf <b>Cust</b> 17,46	om Stage Data (Pr 64 cf Overall x 40.0	<b>ismatic)</b> Listed below (Recalc) 0% Voids
660.50'	15,4	65 cf Cust	om Stage Data (Pr	ismatic) Listed below (Recalc) - Impervious
	22,4	50 cf Total	Available Storage	
n Su	ırf.Area	Inc.Store	Cum.Store	
t)	(sq-ft)	(cubic-feet)	(cubic-feet)	
0	4,366	0	0	
0	4,366	17,464	17,464	
n Su	urf.Area	Inc.Store	Cum.Store	
t)	(sq-ft)	(cubic-feet)	(cubic-feet)	
0	4,366	0	0	
0	4,888	2,314	2,314	
0	6,042	5,465	7,779	
0	7,775	6,909	14,687	
0	7,775	778	15,465	
Routing	Invert	Outlet Dev	ices	
Discarded	656.50'	1.000 in/hi	Exfiltration over \$	Surface area
Primary	658.25'	12.0" Rou	ind Culvert	
Device 2	661.87'	L= 69.0' ( Inlet / Outl n= 0.013 ( <b>24.0'' x 36</b> Limited to	CPP, end-section c et Invert= 658.25' / Corrugated PE, sm <b>0'' Horiz. Orifice/G</b> weir flow at low hea	onforming to fill, Ke= 0.500 657.90' S= 0.0051 '/' Cc= 0.900 ooth interior, Flow Area= 0.79 sf Grate C= 0.600 ads
	Invert 656.50' 660.50' n Su t) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Invert Avail.Sto   656.50' 6,94   660.50' 15,44   22,44 22,44   n Surf.Area   t) (sq-ft)   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 4,366   0 7,775   Routing Invert   Discarded 656.50'   Primary 658.25'   Device 2 661.87'	Invert Avail.Storage Storage   656.50' 6,986 cf Custa   17,46 17,46   660.50' 15,465 cf Custa   22,450 cf Total   n Surf.Area Inc.Store   10 4,366 0   0 4,366 0   0 4,366 17,464   n Surf.Area Inc.Store   10 4,366 0   0 4,366 0   0 4,366 0   0 4,366 0   0 4,366 0   0 4,366 0   0 4,366 0   0 4,366 0   0 4,366 0   0 4,366 0   0 7,775 6,909   0 7,775 778   Routing Invert Outlet Dev   Discarded 656.50' 1.000 in/ht   n= 0.013 (	Invert Avail.Storage Storage Description   656.50' 6,986 cf Custom Stage Data (Pr 17,464 cf Overall x 40.0 660.50' 15,465 cf   660.50' 15,465 cf Custom Stage Data (Pr 22,450 cf Total Available Storage   n Surf.Area Inc.Store Cum.Store   0 4,366 0 0   0 4,366 17,464 17,464   n Surf.Area Inc.Store Cum.Store   0 4,366 0 0   0 4,366 0 0   0 4,366 0 0   0 4,366 0 0   0 4,366 0 0   0 4,366 0 0   0 4,366 0 0   0 6,042 5,465 7,779   0 7,775 778 15,465   Routing Invert Outlet Devices   Discarded 656.50' 1.000 in/hr Exfiltration over 3   Primary

**Discarded OutFlow** Max=0.10 cfs @ 8.55 hrs HW=656.57' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.10 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=656.50' (Free Discharge)

**3=Orifice/Grate** (Controls 0.00 cfs)



## Pond Pond #1: Bioretention Basin

# C-DAT-2002032-PROPOSED HYDROLOCT-Coventry-2002032 24-hr S1 100-yrRainfall=7.90"Prepared by {enter your company name here}Printed 5/4/2021HydroCAD® 10.00-25 s/n 01334© 2019 HydroCAD Software Solutions LLCPage 24

Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-101: Area to Wetland Flow Length=136'	Runoff Area=127,015 sf 12.04% Impervious Runoff Depth=4.84" Slope=0.0145 '/' Tc=25.0 min CN=74 Runoff=9.32 cfs 1.175 af
Subcatchment PDA-201: Area to Ex. CBs	Runoff Area=22,250 sf 26.83% Impervious Runoff Depth=5.88" Flow Length=161' Tc=9.8 min CN=83 Runoff=3.17 cfs 0.250 af
Subcatchment PDA-202: Area to Flow Length=250'	Runoff Area=54,405 sf 50.91% Impervious Runoff Depth=6.12" Slope=0.0100 '/' Tc=16.3 min CN=85 Runoff=6.17 cfs 0.637 af
Subcatchment PDA-203: Building Area	Runoff Area=10,770 sf 100.00% Impervious Runoff Depth=7.66" Tc=5.0 min CN=98 Runoff=2.28 cfs 0.158 af
Reach 1R: Swale to Basin n=0.030 L=	Avg. Flow Depth=0.48' Max Vel=2.28 fps Inflow=2.28 cfs 0.158 af 370.0' S=0.0105 '/' Capacity=9.44 cfs Outflow=2.03 cfs 0.158 af
Reach DP-1: Wetland to Northeast	Inflow=9.32 cfs 1.175 af Outflow=9.32 cfs 1.175 af
Reach DP-2: Ex. CBs in Driveway	Inflow=3.18 cfs 0.552 af Outflow=3.18 cfs 0.552 af
Pond Pond #1: Bioretention Basin Discarded=0.10 c	Peak Elev=662.05' Storage=15,071 cf Inflow=7.69 cfs 0.794 af cfs 0.493 af Primary=2.52 cfs 0.302 af Outflow=2.62 cfs 0.794 af

Total Runoff Area = 4.923 ac Runoff Volume = 2.220 af Average Runoff Depth = 5.41" 72.15% Pervious = 3.552 ac 27.85% Impervious = 1.371 ac

## Summary for Subcatchment PDA-101: Area to Wetland to the Northeast

Runoff = 9.32 cfs @ 12.30 hrs, Volume= 1.175 af, Depth= 4.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

Area (sf)	CN	Description	)	
78,455	69	50-75% Gr	ass cover, l	Fair, HSG B
10,060	79	50-75% Gr	ass cover, l	Fair, HSG C
2,445	60	Woods, Fa	ir, HSG B	
20,760	73	Woods, Fa	ir, HSG C	
13,830	98	Paved park	king, HSG B	3
1,465	98	Paved park	king, HSG C	
127,015	74	Weighted A	Average	
111,720		87.96% Pe	rvious Area	I
15,295		12.04% Im	pervious Ar	ea
Tc Length	n Slop	be Velocity	Capacity	Description
(min) (feet	) (ft/	ft) (ft/sec)	(cfs)	
24.0 100	0.014	5 0.07		Sheet Flow,
				Woods: Light underbrush n= 0.400 P2= 3.31"
1.0 36	6 0.014	0.60		Shallow Concentrated Flow,
				Woodland Kv= 5.0 fps

25.0 136 Total

## Subcatchment PDA-101: Area to Wetland to the Northeast



## Summary for Subcatchment PDA-201: Area to Ex. CBs in Driveway

Runoff = 3.17 cfs @ 12.08 hrs, Volume= 0.250 af, Depth= 5.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

A	rea (sf)	CN I	Description		
	2,580	69	50-75% Gra	ass cover, F	Fair, HSG B
	13,700	79	50-75% Gra	ass cover, F	Fair, HSG C
	0	60	Noods, Fai	r, HSG B	
	0	73	Noods, Fai	r, HSG C	
	910	98 I	⊃aved park	ing, HSG B	
	5,060	98 I	Paved park	ing, HSG C	
	22,250	83	Neighted A	verage	
	16,280	-	73.17% Pei	vious Area	
	5,970		26.83% Imp	pervious Are	ea
-			N / 1 · · ·	0	
	Length	Slope	Velocity	Capacity	Description
(min)	(teet)	(π/π)	(IT/Sec)	(CIS)	
9.3	100	0.0220	0.18		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.31"
0.4	23	0.0174	0.92		Shallow Concentrated Flow,
					Short Grass Pasture Ky= 7 () tos
<b>•</b> • •	~ ~		= 40	o <b>-</b> o	
0.1	38	0.0185	7.16	8.79	Pipe Channel,
0.1	38	0.0185	7.16	8.79	Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
0.1	38	0.0185	7.16	8.79	Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Corrugated PE, smooth interior



## Subcatchment PDA-201: Area to Ex. CBs in Driveway

## Summary for Subcatchment PDA-202: Area to Bioretention Basin

Runoff = 6.17 cfs @ 12.17 hrs, Volume= 0.637 af, Depth= 6.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

A	vrea (sf)	CN I	Description		
	20,135	69	50-75% Gra	ass cover, F	Fair, HSG B
	6,575	79	50-75% Gra	ass cover, F	Fair, HSG C
	0	60	Noods, Fai	ir, HSG B	
	0	73	Noods, Fai	ir, HSG C	
	22,195	98	Paved park	ing, HSG B	6
	5,500	98	<sup>⊃</sup> aved park	ing, HSG C	
	54,405	85	Neighted A	verage	
	26,710	4	49.09% Pei	rvious Area	
	27,695	:	50.91% Imp	pervious Are	ea
_					
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.7	100	0.0100	0.13		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.31"
3.6	150	0.0100	0.70		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps

16.3 250 Total

## Subcatchment PDA-202: Area to Bioretention Basin



## Summary for Subcatchment PDA-203: Building Area

Runoff = 2.28 cfs @ 12.03 hrs, Volume= 0.158 af, Depth= 7.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

	Area (sf)	CN	Description			
	0	69	50-75% Grass	cover, F	<sup>-</sup> air, HSG B	
	0	79	50-75% Grass	cover, F	<sup>-</sup> air, HSG C	
	0	60	Woods, Fair, F	ISG B		
	0	73	Woods, Fair, F	ISG C		
	10,770	98	Paved parking	, HSG B		
	0	98	Paved parking	, HSG C	;	
	10,770	98	Weighted Ave	rage		
	10,770		100.00% Impe	rvious A	rea	
Tc	Length	Slop	e Velocity C	apacity	Description	
(min)	(feet)	(ft/	t) (ft/sec)	(cfs)		
5.0					Direct Entry,	

### Subcatchment PDA-203: Building Area



C-DAT-2002032-PROPOSED HYDROLOCT-Coventry-2002032 24-hr S1 100-yrRainfall=7.90"Prepared by {enter your company name here}Printed 5/4/2021HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD Software Solutions LLCPage 30

## Summary for Reach 1R: Swale to Basin



## Summary for Reach DP-1: Wetland to Northeast

Inflow Are	a =	2.916 ac, 1	2.04% Impervious	, Inflow Depth =	4.84	for 100-yr event
Inflow	=	9.32 cfs @	12.30 hrs, Volum	e= 1.175	af	
Outflow	=	9.32 cfs @	12.30 hrs, Volum	e= 1.175	af, A	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



## **Reach DP-1: Wetland to Northeast**

## Summary for Reach DP-2: Ex. CBs in Driveway

Inflow Ar	ea =	2.007 ac, 50.83% Impervious, Inf	low Depth = 3.30" for	<sup>r</sup> 100-yr event
Inflow	=	3.18 cfs @ 12.49 hrs, Volume=	0.552 af	
Outflow	=	3.18 cfs @ 12.49 hrs, Volume=	0.552 af, Atten=	0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



## Reach DP-2: Ex. CBs in Driveway

## Summary for Pond Pond #1: Bioretention Basin

Inflow Area	=	1.496 ac, 5	9.02% Imp	ervious, Inflow D	Depth = 6.3	37" for 100-	yr event
Inflow	=	7.69 cfs @	12.14 hrs,	Volume=	0.794 af		
Outflow	=	2.62 cfs @	12.50 hrs,	Volume=	0.794 af,	Atten= 66%,	Lag= 21.7 min
Discarded	=	0.10 cfs @	5.84 hrs,	Volume=	0.493 af		
Primary	=	2.52 cfs @	12.50 hrs,	Volume=	0.302 af		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 662.05' @ 12.50 hrs Surf.Area= 4,366 sf Storage= 15,071 cf

Plug-Flow detention time= 786.5 min calculated for 0.794 af (100% of inflow) Center-of-Mass det. time= 786.5 min (1,586.2 - 799.7)

Volume	Inve	ert Ava	il.Storage	e Storaç	ge Description	
#1	656.5	50'	6,986 c	f <b>Custo</b> 17,46	m Stage Data (Pr 4 cf Overall x 40.0	<b>ʻismatic)</b> Listed below (Recalc) 0% Voids
#2	660.5	50'	15,465 c	f Custo	om Stage Data (Pr	ismatic) Listed below (Recalc) - Impervious
			22,450 c	f Total	Available Storage	
Elevatio	on	Surf.Area	I	nc.Store	Cum.Store	
(fee	et)	(sq-ft)	(CL	ibic-feet)	(cubic-feet)	
656.5	50	4,366		0	0	
660.5	50	4,366		17,464	17,464	
Elevatio	on	Surf.Area	ļ	nc.Store	Cum.Store	
(fee	et)	(sq-ft)	(CL	ibic-feet)	(cubic-feet)	
660.5	50	4,366		0	0	
661.0	00	4,888		2,314	2,314	
662.0	00	6,042		5,465	7,779	
663.0	00	7,775		6,909	14,687	
663.1	10	7,775		778	15,465	
Device	Routing	Ir	vert O	utlet Devi	ces	
#1	Discarde	d 650	6.50' <b>1.</b>	000 in/hr	Exfiltration over	Surface area
#2	Primary	658	3.25' <b>12</b>	2.0" Roui	nd Culvert	
			L=	= 69.0' C	PP, end-section c	onforming to fill, Ke= 0.500
			In	let / Outle	t Invert= 658.25' /	657.90' S= 0.0051 '/' Cc= 0.900
			n=	= 0.013 C	corrugated PE, sm	ooth interior, Flow Area= 0.79 sf
#3	Device 2	66	1.87' <b>2</b> 4	1.0" x 36.0	" Horiz. Orifice/O	Frate C= 0.600
			LI	mited to v	veir flow at low he	ads

**Discarded OutFlow** Max=0.10 cfs @ 5.84 hrs HW=656.57' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.10 cfs)

**Primary OutFlow** Max=2.50 cfs @ 12.50 hrs HW=662.05' (Free Discharge) **2=Culvert** (Passes 2.50 cfs of 5.84 cfs potential flow)

**3=Orifice/Grate** (Weir Controls 2.50 cfs @ 1.39 fps)



## Pond Pond #1: Bioretention Basin



## APPENDIX D

## WATER QUALITY CALCUATIONS CTDEEP Water Quality Volume Calculations

CTDEEP Water Quality Volume Calculations Bioretention Basin Calculation Groundwater Recharge Calculation Treatment Train Efficiency Worksheet

#### Water Quality Calculations

#### Determine Water Quality Volume

From CT 2004 Stormwater Quality Manual:

$$WQV = \frac{(1'')(R)(A)}{12}$$

R = 0.05 + 0.009(I)

WQV = water quality volume (ac-ft) R = volumetric runoff coefficient I = percent impervious cover A = site area in acres

WQv = Calculated Water Quality Volume

Area		Total Area Impervious Area		Impervious Cover	Volumetric Runoff Coefficient	Water Qua (W	Water Quality Volume (WQV)		Proposed Water Quality Volume (WQV)		
ID		ac	ft <sup>2</sup>	ac	ft <sup>2</sup>	%	R	acre-feet	ft <sup>3</sup>	acre-feet	ft <sup>3</sup>
Area to Bioretention Basin	PDA 202/PDA 203	1.496	65,175	0.883	38,465	59.02	0.581	0.072	3,136	0.117	5,079

## **Bioretention Basin Calculations**

#### Surface area of the Bioretention System SA=(WQv)/hf

WQv = Calculated Water Quality Volume hf=depth of ponding above soil media in feet)

		Water Quality				
		Volume	Depth of	Required Surface	Surface Area	WQV Provided in
		Required	Ponding	Area	Provided	Ponded Depth
		(CF)	(FT)	(SF)	(SF)	(CF)
Bioretention Basin #1	PDA 202/PDA 203	3,136	1.37	2,289	4,366	5,079

#### **Groundwater Recharge Volume Calculations**

#### Groundwater Recharge Volume

From CT 2004 Stormwater Quality Manual:

$$GVR = \frac{(D)(A)(I)}{12}$$

GRV Groundwater Recharge Volume (ac-ft) D = Depth of Runoff to be Recharged (table 7-4) A = site area in acres I = impervious cover (decimal) WQv = Calculated Water Quality Volume

4366

1.37

A											Ì			
Total Site Area	Sito Arc	Site Area by NRCS Hydrologic Soil Group				han a minure Oracen has NIDOO I hadrada aire Orill Oracen			Si	te Impervious	ness (Decim	el)	GRV	Potential Recharge
	Sile Ale		Tyurologic Sol	Gloup	Impervious Cover by NRCS Hydrologic Soli Group		by NRCS Hydrologic Soil Group			Required	Pond Volumes			
(AC)	A	В	С	D	A	В	С	D	А	В	С	D	(ac-ft)	Proposed (ac-ft)
1.87	0.00	1.46	0.41	0.00	0.00	0.76	0.13	0.00	0.00	0.41	0.07	0.00	0.013	0.117

Table 7-4 Groundwater Recharge Depth								
NRCS Hydrologic Soil Group	Average Annual Recharge	Groundwater Recharge Depth (D)						
A	18 inches/year	0.4 inches						
В	12 inches/year	0.75 inches						
C	6 inches/year	0.10 inches						
D	3 inches/year	0 inches (waived)						

Source: MADEP, 1997. NRCS - Natural Resources Conservation Service

Best Manager	nent Practice (BMP) Treatment Train Efficier	ncy Worksheet							
Prepared for: Proposed Retail Development 1100 Boston Turnpike Bolton, Connecticut									
Prepared by: BL Companies 100 Constitution Plaza, 10th Floor Hartford Connecticut									
Date prepared: April 2, 2021									
Et=[1-(1-E1)(1-E2)(1-E3)(1-E4)(1-E?)]*100	BMP BMP Description   E1 Impervious Surface Sweeping***   E2 Bioretention Basin	<u>Type of Treatment</u> Secondary (conventional) Primary	Efficiency Rate % 10 90	<u>BMP</u> Impervious Surface Sweeping*** Bioretention Basin	<u>Type of Treatment</u> secondary (conventional) Primary	<u>TSS Removal</u> <u>Rate</u> 0.10 0.9	<u>Starting TSS</u> <u>Load</u> 1.00 0.90	<u>Amount</u> <u>Removed</u> 0.10 0.81	<u>Remaining</u> <u>Load</u> 0.90 0.09
Overall Treatment Train Efficiency (Et)=	91 % Total Suspended Solids (TSS) Remova			Overall Treatment Train Efficiency (%					91
* 80% require per CT DEP ** Manufacturers claim 80% TSS removal *** Schueler 1996 & EPA 1993 **** University of New Hampshire									

#### TSS Removal Rates (adapted from Schueler, 1996, & EPA, 1993)

· · · · · · · · · · · · · · · · · · ·	<u> </u>		4
BMP List	Design	Range of	Brief Design Requirements
	Rate	Average TSS	
	I	Removal Rates	
Extended Detention Pond	70%	60-80%	Sediment forebay
	I		
Wet Pond (a)	70%	60-80%	Sediment forebay
	L		
Constructed Wetland (b)	80%	65-80%	Designed to infiltrate or retain
JJ	L		
Water Quality Swale	70%	60-80%	Designed to infiltrate or retain
<u>اا</u>	1		
Infiltration Trench	80%	75-80%	Pretreatment critical
<u>اا</u>	1		
Infiltration Basin	80%	75-80%	Pretreatment critical
.l I	1	(predicted)	
Dry Well	80%	80% (predicted)	Rooftop runoff
. · · · · · · · · · · · · · · · · · · ·	1	· · ·	1
	1		(uncontaminated only)
	1		(
Sand Filter (c)	80%	80%	Pretreatment
Organic Filter (d)	80%	80%+	Pretreatment
Water Quality Inlet	25%	15-35% w/	Off-line only; 0.1" minimum Water Quality Volume (WQV) storage
1 1	1		
4 1	1	cleanout	
Sediment Trap (Forebay)	25%	25% w/	Storm flows for 2-year event must not cause erosion; 0.1" minimum WQV storage
1	1	cleanout	······································
Drainage Channel	25%	25%	Check dams: non-erosive for 2-yr.
Diamage Channel	2270	2070	check dams, hon-closive ic. 2 yr.
1 1	1		
Deep Sump and Hooded Catch	25%	25% w/	Deep sump general rule = 4 x pipe diameter or 4.0' for pipes 18" or less
Basin		cleanout	beep sump general rate of the product of the produc
Street Sweeping	10%	10%	Discretionary non-structural credit, must be part of approved plan
Successicoping		1070	Districtionally non-structural erean, must be part of approved pain



## APPENDIX E

SUBSURFACE SOIL INVESTIGATION LOGS Test Pit Logs Falling Head Permeability Test Logs





		T	EST PIT I	FIELD	LOG			
PERSON	NEL PRESENT		EXCAVATION	EQUIPMEN	νT			
Cody L'Heureux-	BL Companies	Contractor				Ground Surfac	e Elevation	662.50
-	-	Operator				Datum	NAVD	88
		Make		Model		Temperature	54	
		Bucket Capacity		Reach		Weather	Cloudy w	/ Rain
							Calification of	<del></del>
Depth		SOIL D	DESCRIPTION			Excav. Effort	Boulder Data	Remark No.
0"-2"	Topsoil					Е		
2"-60"	Dark brown coarse sa	nd with trace cobbl	es			Е	TR C	
60"-120"	Dark brown silty sand	1				Е		1
		Bottom of T	'est Pit at 120" (	(10')				
REMARKS:								
1. Ground wate 2. Bedrock was	r was observed at 8'. not observed.							
TEST PIT PL	AN			LEG	END			
-O+ North	COBBLES Size Range Classification 3" - 12" 12" - 24 24" - 36" 36" and Larger	Letter Designation Cobble (C) Small (S) Medium (M) Large (L)	PROPORTIC (QUANTITAT) TRACE (TR) LITTLE (LI) SOME (SO) MANY (MA)	0-10% 10-20% 20-35% 35-50%	QUALITATIVE TERMS OCCASIONAL FEW FREQUENT NUMEROUS	E - Easy M - Moderate D - Difficult	Observed Depth to Groundwater	T

355 Research Parkway Meriden, CT 06450 Tel.(203) 630-1406 Fax (203) 630-2615

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		T	EST PIT FIELD	LOG			
PERSON	NEL PRESENT	]	EXCAVATION EQUIPME	ENT			
Cody L'Heureux	- BL Companies	Contractor			Ground Surfac	e Elevation	662.30
-	•	Operator			Datum	NAVD	88
		Make	Model		Temperature	54	
		Bucket Capacity	Reach		Weather	Cloudy w	/ Rain
						0.111 1	1
Depth		SOIL D	DESCRIPTION		Excav. Effort	Boulder Data	Remark No.
0"-6"	Topsoil				Е		
6"-72"	Dark brown coarse	sand with trace cobbl	es		Е	TR C	
72"-120"	Dark brown silty sa	nd			Е		1
		Bottom of T	'est Pit at 120" (10')				
REMARKS: 1. Ground wate 2. Bedrock was	er was observed at 8'.						
TEST PIT PL	AN			GEND			
North	COBBLE Size Range Classification 3" - 12" 12" - 24 24" - 36" 36" and Large	Letter Designation Cobble (C) Small (S) Medium (M) er Large (L)	PROPORTIONS USED (QUANTITATIVE TERMS) TRACE (TR) 0-10% LITTLE (LI) 10-20% SOME (SO) 20-35% MANY (MA) 35-50%	QUALITATIVE TERMS OCCASIONAL FEW FREQUENT NUMEROUS	E - Easy M - Moderate D - Difficult	Observed Depth to Groundwater	Г 

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		<b>T</b> ]	EST PIT	FIELD	LOG	_		
PERSON	NEL PRESENT		EXCAVATION	EQUIPME	NT			
Cody L'Heureux-	- BL Companies	Contractor				Ground Surfa	ce Elevation	661.90
5	1	Operator				Datum	NAVD	88
		Make		Model		Temperature	54	
		Bucket Capacity		Reach		Weather	Cloudy w	/ Rain
				-				
Depth		SOIL I	DESCRIPTION			Excav. Effort	Cobble and Boulder Data	Remark No.
0"-6"	Topsoil					Е		
6"-66"	Dark brown coarse s	and with trace cobb!	les			E	TR C	
66"-120"	Dark brown silty san	ıd				Е		1
		Bottom of T	est Pit at 120"	(10')				
REMARKS:								
1. Ground wate 2. Bedrock was	er was observed at 7'.							
TEST PIT PL	AN			LEO	GEND			
North	COBBLES Size Range Classification 3" - 12" 12" - 24 24" - 36" 36" and Larger	AND BOULDERS Letter Designation Cobble (C) Small (S) Medium (M) Large (L)	PROPORTIC (QUANTITAT) TRACE (TR) LITTLE (LI) SOME (SO) MANY (MA)	0-10% 10-20% 20-35% 35-50%	QUALITATIVE TERMS OCCASIONAL FEW FREQUENT NUMEROUS	E - Easy M - Moderate D - Difficult	Observed Depth to Groundwater	tr 

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#### FALLING HEAD PERMEABILITY TEST

PROJECT:	Proposed Retail Development	PROJECT	#2002032	BY:	C.J.L.
	Bolton, CT	DATE:	5/4/2021		

TEST PIT # 1

#### SAMPLE TP-1

SAMPLE LENGTH:	4.50	in.	
SAMPLE DEPTH (BELC	OW EG):		6.00 ft

presoak start: 10:00 am presoak finish: 10:30 am



## $K = \frac{(H1 - H2) \times L}{t \times (H1 + H2)/2}$

Time	H1	H2	H1 - H2	(H1 + H2)/2	K	K	
(min.)	(in.)	(in.)			(in/min.)	<u>(ft./day)</u>	
0.000	6.500	6.500	0.000	6.500	-	-	
5.000	6.500	6.260	0.240	6.380	0.034	4.063	
10.000	6.500	5.960	0.540	6.230	0.039	4.681	
15.000	6.500	5.720	0.780	6.110	0.038	4.596	
20.000	6.500	5.540	0.960	6.020	0.036	4.306	
25.000	6.500	5.420	1.080	5.960	0.033	3.914	
30.000	6.500	5.300	1.200	5.900	0.031	3.661	
35.000	6.500	5.060	1.440	5.780	0.032	3.844	
40.000	6.500	4.880	1.620	5.690	0.032	3.844	
45.000	6.500	4.680	1.820	5.590	0.033	3.907	
50.000	6.500	4.500	2.000	5.500	0.033	3.927	
					Average=	4.074	ft/day
					or	2.04	in/hr

#### FALLING HEAD PERMEABILITY TEST

PROJECT:	Proposed Retail Development	PROJECT #	#2002032	BY:	C.J.L.
	Bolton, CT	DATE:	5/4/2021		

TEST PIT # 1

#### SAMPLE TP-2

SAMPLE L	ENGTH:	4.50	in.	
SAMPLE DEPTH (BELOW EG):				6.00 ft

presoak start: 10:00 am presoak finish: 10:30 am





Time	H1	H2	H1 - H2	(H1 + H2)/2	K	K	
(min.)	(in.)	(in.)			(in/min.)	(ft./day)	
0.000	6.500	6.500	0.000	6.500	-	-	
5.000	6.500	6.170	0.330	6.335	0.047	5.626	
10.000	6.500	5.880	0.620	6.190	0.045	5.409	
15.000	6.500	5.650	0.850	6.075	0.042	5.037	
20.000	6.500	5.300	1.200	5.900	0.046	5.492	
25.000	6.500	5.060	1.440	5.780	0.045	5.381	
30.000	6.500	4.680	1.820	5.590	0.049	5.860	
35.000	6.500	4.500	2.000	5.500	0.047	5.610	
40.000	6.500	4.300	2.200	5.400	0.046	5.500	
45.000	6.500	4.000	2.500	5.250	0.048	5.714	
					Average=	5.514	ft/day
					or	2.76	in/hr

#### FALLING HEAD PERMEABILITY TEST

PROJECT:	Proposed Retail Development	PROJECT #	#2002032	BY:	C.J.L.
	Bolton, CT	DATE:	5/4/2021		

TEST PIT # 1

#### SAMPLE TP-3

SAMPLE LENGTH:	4.00	in.	
SAMPLE DEPTH (BEL	.OW EG):		6.00 ft

presoak start: 10:00 am presoak finish: 10:30 am



## $K = \frac{(H1 - H2) \times L}{t \times (H1 + H2)/2}$

Time	H1	H2	H1 - H2	(H1 + H2)/2	K	K	
(min.)	(in.)	(in.)			(in/min.)	(ft./day)	
0.000	6.500	6.500	0.000	6.500	-	-	
5.000	6.500	6.180	0.320	6.340	0.040	4.845	
10.000	6.500	5.540	0.960	6.020	0.064	7.654	
15.000	6.500	5.060	1.440	5.780	0.066	7.972	
20.000	6.500	4.280	2.220	5.390	0.082	9.885	
25.000	6.500	4.220	2.280	5.360	0.068	8.167	
30.000	6.500	4.000	2.500	5.250	0.063	7.619	
					Average=	7.691	ft/day
					or	3.85	in/hr



May 5, 2021

Patrice L. Carson, AICP, Director of Community Development Town of Bolton 222 Bolton Center Road Bolton, CT 06043

Re: Special Permit Application 2-Lot Subdivision Application Proposed Retail Development 1100 Boston Turnpike

Dear Ms. Carson:

We are in receipt of engineering comments dated April 26, 2021, from Nathan L. Jacobson & Associates, Inc., and your comments dated May 3, 2021, regarding the project referenced above. Our responses below are shown in *bold italic* text. We have provided the following additional information documenting our responses:

- Land Development Plans, revised to May 5, 2021
- Stormwater Management Narrative and Hydrologic Calculations, dated May 5, 2021

## NATHAN L. JACOBSON & ASSOCIATES, INC.

1. Referral should be made to the Connecticut Department of Transportation (CTDOT) for work proposed within the State right-of-way.

Response: Reference to CTDOT requirements for permanent pavement repair in Route 44 has been included on the revised Site Plan sheet SP-1. The CTDOT standard details for trench repair are included on Details Sheet DN-6.

2. An application should be made to the Bolton Lakes Regional Water Pollution Authority (BLRWPCA) prior to the installation of the pressure line and sewage pump station.

## *Response:* Acknowledged. An application to the BLRWPCA has been initiated; coordination is ongoing.

3. We would recommend that an underdrain be placed along the centerline of the proposed water quality basin to help to drain the basin between rain events and prevent problems with standing water.



Response: A 4" perforated HDPE underdrain has been added to the water quality basin. A raised underdrain system is being utilized to create an internal water storage zone within the bioretention soil media. This configuration will allow for a 6" depth of submerged bioretention soil to provide enhanced nitrogen removal. Please refer to the updated Grading and Drainage Plan sheet GD-1 and Details Sheet DN-3.

4. An existing conditions model should be provided for the subarea that contributes to the water quality basin on the Bolton Dental site. From the data provided in the Stormwater Management Report, it appears that flows being sent to the existing basin will be minimal, but it should be quantified.

Response: Acknowledged. A supplementary Stormwater Management Narrative and Hydrologic Calculations letter has been provided with this submission. The letter details and compares the existing and proposed condition peak flows directed offsite. As indicated in the letter, peak flow directed to the Bolton Dental site is matched in the 2, 10, and 100-year storm events.

5. The available Water Quality Volume (WQv) should be calculated as the volume of the basin up to the lowest proposed outlet elevation. In this regard, the available storage should be calculated only up to the top of frame of the proposed outlet structure. The data provided in the Stormwater Management Report indicates that the basin still provides adequate WQv even with the reduced available storage. The "Proposed Water Quality Volume (WQv)" column of the WQv table should be revised for accuracy.

Response: The Bioretention Basin Calculations sheet has been revised to indicate the correct depth of ponding of 1.37' between the revised outlet structure frame (661.87') and the revised bottom of the bioretention basin (660.50'). The WQV provided in the ponded depth is 5,079 CF. The updated Stormwater Quality Calculations can be seen in Appendix D of the supplementary Stormwater Management Narrative and Hydrologic Calculations letter included with this submission.

6. Test pits should be performed within the area of the proposed stormwater basin to determine if rock or high groundwater will conflict with the soil media and drainage layers below the proposed basin floor elevation.

Response: Test pits were performed on site on 5/4/2021. A total of three test pits, spaces 50' on center, were observed spanning the location of the proposed bioretention basin. Test pit locations have been added to the Land Development Plans included with this submission.

The test pits indicate that the site soils consist of dark brown sand within the upper 5' to 6' below existing grade, transitioning to sand with silt in below 6'. Groundwater was observed in all three pits ranging in depth from 7' to 8' below grade. Test pit 1 (TP-1) represents the highest potential groundwater table, with water located at elevation 654.50. To provide the 2' minimum required vertical separation from groundwater to



the bottom of the bioretention media, the bioretention basin elevation has been raised by 0.50'. The bottom of the  $\frac{3}{4}$ " stone layer is now proposed at elevation 656.50, and the bottom of the above-ground basin storage is now 660.50.

Falling head soil permeability testing was also performed for each test pit. Results of permeability testing ranged from 2.04 in/hr to 3.85 in/hr. The infiltration rate of 1 in/hr used in the hydrologic modeling conservatively represents the minimum field-measured rate of 2.04 in/hr reduced by half to accommodate a clogging factor of safety.

### COMMUNITY DEVELOPMENT STAFF ANALYSIS – 2-LOT SUBDIVISION APPLICATION

1. PZC determined no Public Hearing would be held and was not required by CGS.

### Response: Acknowledged.

2. A feasibility plan needs to be shown for lot #2 which would include proposed locations of a building, parking, and general site layout.

Response: A feasibility plan indicating potential locations of a building, parking and drive aisle areas, and other general site layout items has been provided in the Land Development Plans. Please refer to Master Plan sheet MP-1.

3. Once Street Numbers are assigned and approved by the Town, they should be shown on the Plan.

## *Response:* Acknowledged, appropriate street numbers will be added to the plans upon approval.

4. Section 16A.3.x. – Buildings and Structures: Architectural and Design Requirements & Section 16B.4.1. – Architectural Character, Historic Preservation, Site Design. The Commission needs to determine if the design of the proposed building is adequate to meet these standards. If the Commission's intention along this corridor is to preserve the residential-type character and create transitions to existing residential neighborhoods, this proposal seems to accomplish that. Staff feels the applicant has paid particular attention to keeping all activity (no lighting, windows, etc.) away from the west side of the building to keep from interfering with the residences on North Road.

Response: Acknowledged, please note that additional brick banding and faux windows have been added to the northern side of the building in an effort to increase visual aesthetics while mitigating disturbance to the residential abutters. Please refer to the updated architectural elevations provided in the Land Development Plan set included with this submission.



5. Subdivision Regulations Section 4 – Open Space – The Commission needs to declare on the record that the conservation easement on the west side of the property satisfies the Open Space requirement of the Subdivision Regulations.

## Response: Acknowledged.

<u>COMMUNITY DEVELOPMENT STAFF ANALYSIS – SPECIAL PERMIT APPLICATION</u> The plans appear to meet Town Regulations. The following items may require additional information:

1. The PZC will hold a Public Hearing on May 5, 2021 as required by CGS.

## Response: Acknowledged.

2. Once Street Numbers are assigned and approved by the Town, they should be shown on the Plan.

## *Response:* Acknowledged, appropriate street numbers will be added to the plans upon approval.

3. The Town Engineer's comments need to be addressed.

## *Response: The Town Engineer's comments have been addressed, please refer to the Nathan L. Jacobson & Associates, Inc. portion of this response letter.*

4. Section 16A.3.x. – Buildings and Structures: Architectural and Design Requirements & Section 16B.4.1. – Architectural Character, Historic Preservation, Site Design. The Commission needs to determine if the design of the proposed building is adequate to meet these standards. If the Commission's intention along this corridor is to preserve the residential-type character and create transitions to existing residential neighborhoods, this proposal seems to accomplish that. Staff feels the applicant has paid particular attention to keeping all activity (no lighting, windows, etc.) away from the west side of the building to keep from interfering with the residences on North Road.

## Response: Acknowledged, please note that additional brick banding and faux windows have been added to the northern side of the building in an effort to increase visual aesthetics while mitigating disturbance to the residential abutters. Please refer to the updated architectural elevations provided in the Land Development Plan set included with this submission.

5. Section 16A.4.d. – Notices – Statutory notices have been published on the town's website, and the applicant has been provided with a sign for posting. The applicant has provided an affidavit for the posting of a sign.

## Response: Acknowledged.



### General Revision Notes

- 1. A conduit sleeve under the proposed development driveway, within the access and utility easement area, is now shown on the revised Site Utility Plan sheet SU-1. The conduit sleeve may be used for future sign conduit installation to Lot 2.
- 2. Additional brick banding and faux windows have been added to the northern side of the building in an effort to increase visual aesthetics while mitigating disturbance to the residential abutters. Please refer to the updated Conceptual Elevations sheet by BKA Architects dated May 3, 2021 provided in the Land Development Plan set included with this submission.
- 3. Screening vegetation to be planted between the proposed building and the residential abutters has been relocated to align with the top of the existing earthen berm, rather than being planted on the sloped portion of the berm, to maximize screening efficiency. Please refer to the updated Landscape Plan sheet LL-1.
- 4. The sanitary sewer service connection has been relocated to the force main along Boston Turnpike, greatly reducing the length of sanitary lateral, at the recommendation of the BLRWPCA. The Land Development Plans have been updated to indicate approximate connection invert to the main and required lateral kit installation. The plans also indicate the location of a proposed 20' wide sanitary sewer easement in favor of the BLRWPCA on the project parcel up to and including the sanitary sewer pump station. Please refer to the revised Site Utility Plan sheet SU-1.
- 5. The developer will provide a site work completion bond and sewer bond as required.
- 6. The property owner's signature will be provided on required applications upon receipt.
- 7. Reponses to comments from the Eastern Highlands Health district received on 5/5/2021 are forthcoming.

We trust this addresses your concerns. Should you require additional information, please contact me at 860-760-1908.

Sincerely,

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Kimberly M. Masiuk, P.E. Senior Project Manager